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Impacts of Potential Development on Groundwater Resources in the Community of Cutter, New Mexico.

Chance Coats

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Impacts of Potential Development on Groundwater Resources in the Community of Cutter, New Mexico.

By

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Committee

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A Professional Project Submitted in Partial Fulfillment of the Requirements for the Degree of

Master of Water Resources

Hydroscience Concentration

Water Resources Program

The University of New Mexico

Albuquerque, New Mexico

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Committee Approval

The Master of Water Resources Professional Project Report of **Chance Coats**, entitled **Impacts of Potential Development on Groundwater Resources in the Community of Cutter, New Mexico**, is approved by the committee:

Chair

Date

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To my family, you are my anchor in life that keeps me grounded and have supported me all the way. I wouldn't be here without you. To my Dad, Uncle, and late Grandfather, thank you for bringing me up in a world of pumps and water. To my wife, thank you so much for your support and patience through graduate school. I love you. And finally, to my little girl to be born in March, you already inspire me. I can't wait for you to get here.

Abstract

Commercial space flight from New Mexico is soon to become a reality. With Spaceport America soon to be in full operation, economic growth is expected to increase in the Jornada del Muerto Basin. A potential site for future development is located five miles north of the Spaceport, in the community of Cutter, New Mexico. This project discusses the groundwater resources available beneath the community of Cutter. Data from a pumping test performed on well LRG-10140 along with groundwater level measurements from the New Mexico Office of the State Engineer (NMOSE) and water level data provided by John Shomaker and Associates, Inc. (JSAI) was used to evaluate the system.

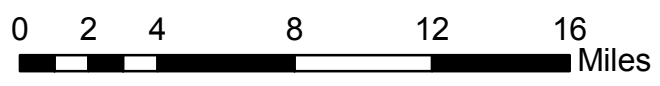
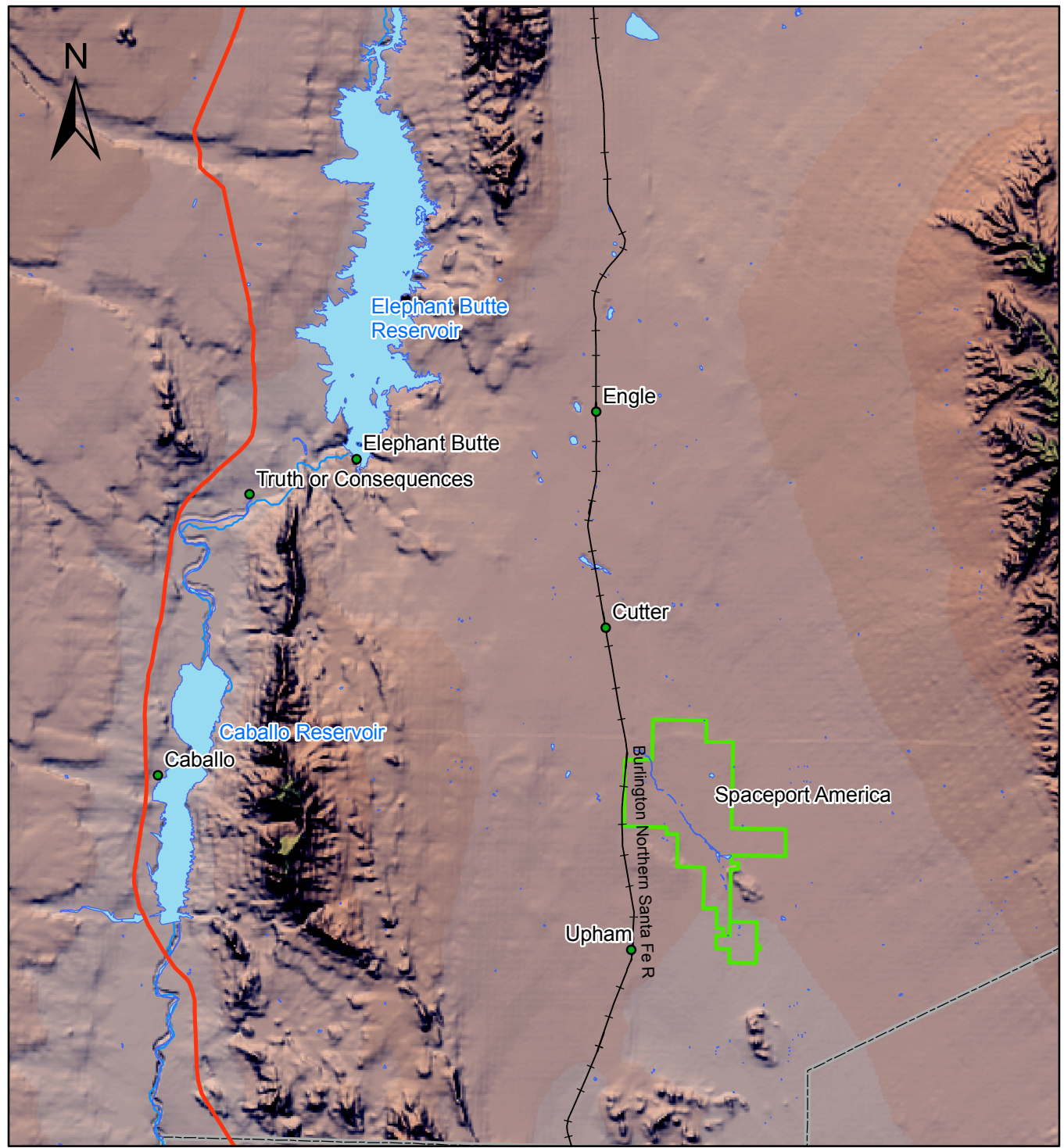
The hydrogeology of the system beneath Cutter is best defined as a fractured tub. It is bound on the east side by the Jornada Draw Fault and the south by series of Tertiary dikes. Recharge estimated at 80 ac-ft/yr (acre feet per year) comes from the northwest and flows to the southeast. The fractured sandstone units of the Mesaverde Formation provide for a low storage coefficient of 0.005 using the specific yield approach for calculating storage.

The Big Sky Village Subdivision makes up the majority of developed land within the study area. Current demand in this area is estimated at 40.9 ac-ft/yr. The findings of this study estimate the system beneath Cutter can provide for a population of 200 people at a demand of 200 gallons per capita per day. At this population, the demand overtakes the recharge into the system and the aquifer is not sustainable. Drawdown at this demand will provide a groundwater supply for the community for 40 years into the future.

Introduction

Spaceport America (Spaceport) was constructed to support commercial spaceflight in New Mexico. This facility will be the first of its kind and provides for both manned and unmanned launches. With this new industry, jobs, economic growth, and commerce are expected to increase in the vicinity of the Spaceport. One nearby community is Cutter, New Mexico. Cutter is located approximately 25 miles southeast of Truth or Consequences, New Mexico, in the Jornada del Muerto Basin (Jornada) and lies only a few miles north of the Spaceport (Fig. 1, pg.2). It is the closest community to the Spaceport and may experience growth due to its proximity to the facility. However, growth is likely to be constrained by water availability.

The main focus of this project was to evaluate the groundwater resources beneath the community of Cutter. A preliminary evaluation was conducted for the use of the Romero well (LRG-10140) by contractors for temporary water supply during the early construction phases of the Spaceport. Early pumping and analysis raised questions about the resources in the aquifer. Drawdown in nearby wells was greater than first estimated. This suggested that there was less water in storage than was originally anticipated. While the amount of water pumped from LRG-10140 during construction far exceeds the usual diversions within this system, it does suggest that groundwater resources are limited. The objective of this project was to determine the availability of groundwater resources beneath Cutter, NM.



Explanation	
●	Town
—	I-25
—+—	Railroad
■	Lake
—	Rio Grande
□	Spaceport Boundary
□	County Line

Figure 1. Map showing location of Spaceport America and Cutter, NM

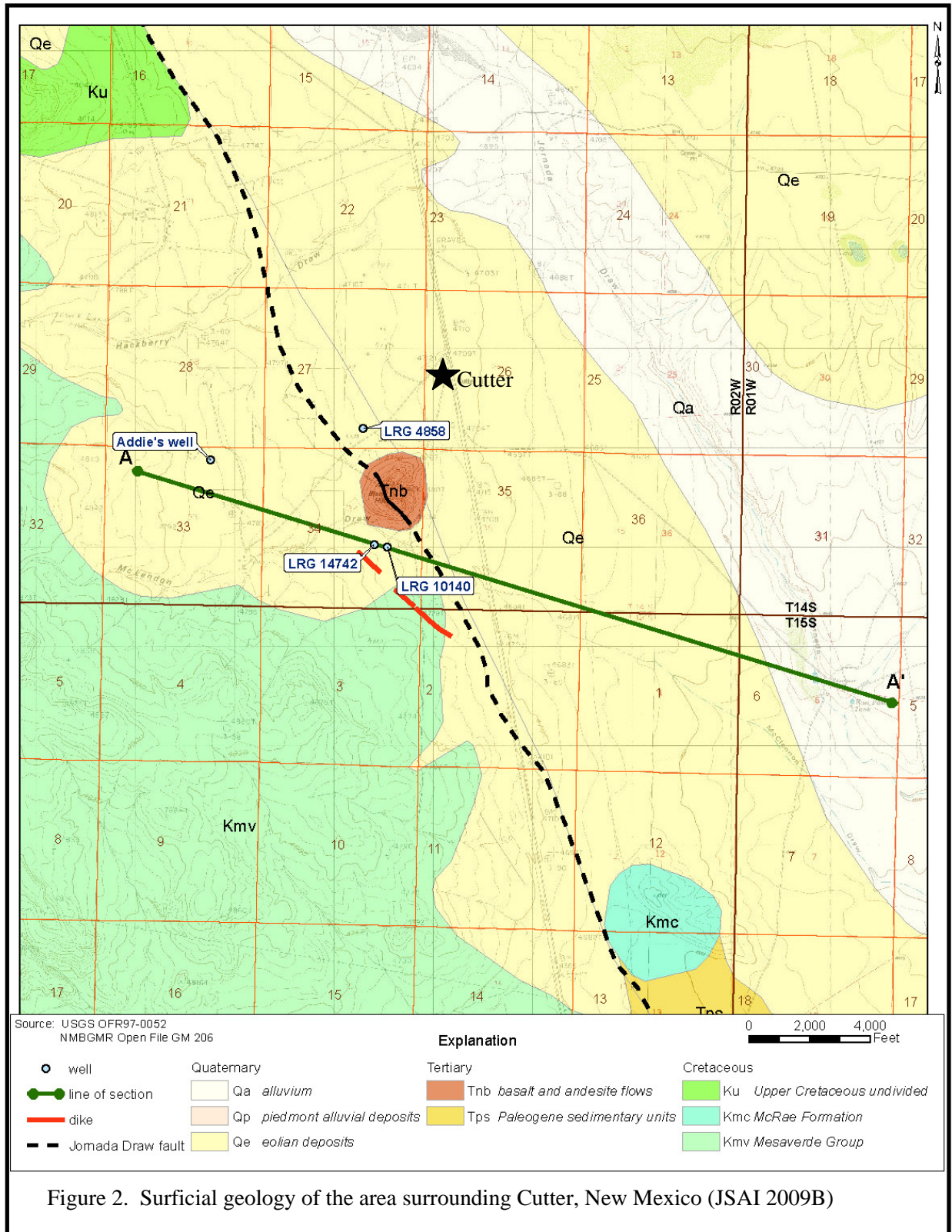
Geologic Setting

The well log for LRG-10140 (Appendix A) describes alternating beds of shale and sandstone. Water is contained within the sandstone beds of the Mesaverde Formation, which are locally fractured. Recharge likely occurs west of LRG-10140, where sandstone beds crop out at land surface, and possibly from fractured volcanic rocks associated with Black Hill (Fig. 2, pg.7) (JSAI, 2009B).

The sandstone beds of the Mesaverde Formation are locally fractured due to the intrusion at Black Hill and faulting. The Jornada Draw fault is classified as a normal fault (Seager and Mack, 1995). Beds of the Mesaverde Formation dip 5 degrees to 15 degrees from east to southeast (Fig. 3 pg. 8). The thickness of the Mesaverde Formation mapped at the LRG-10140 site is 1,000 ft (Seager, 2007). Regional groundwater flows south to southeast (JSAI, 2009B).

Fault zones can act as either barriers to groundwater flow or as groundwater conduits, depending on the nature of the material in the fault zone. Impounding faults can occur in unconsolidated materials with clay present, as well as in sedimentary rocks where interbedded shales, which normally would not hinder lateral groundwater flow, can be smeared along the fault by drag folds (Fetter, 1988). The Jornada Draw fault acts as a barrier to groundwater flow in this localized system as will be discussed later.

Flow in fractured formation is difficult to understand and analyze. For one, the network and size of fractures are often not possible to predict unless surface manifestations are observed. Furthermore, flow in some larger fractures is turbulent as opposed to laminar, so Darcy's law should not be applied to these (Fitts, 2002). While it is safe to assume the majority of larger fractures within this area of the Mesaverde Formation are orientated parallel to the Jornada Draw fault (Fig. 2, pg. 7), their size and frequency are impossible to predict.



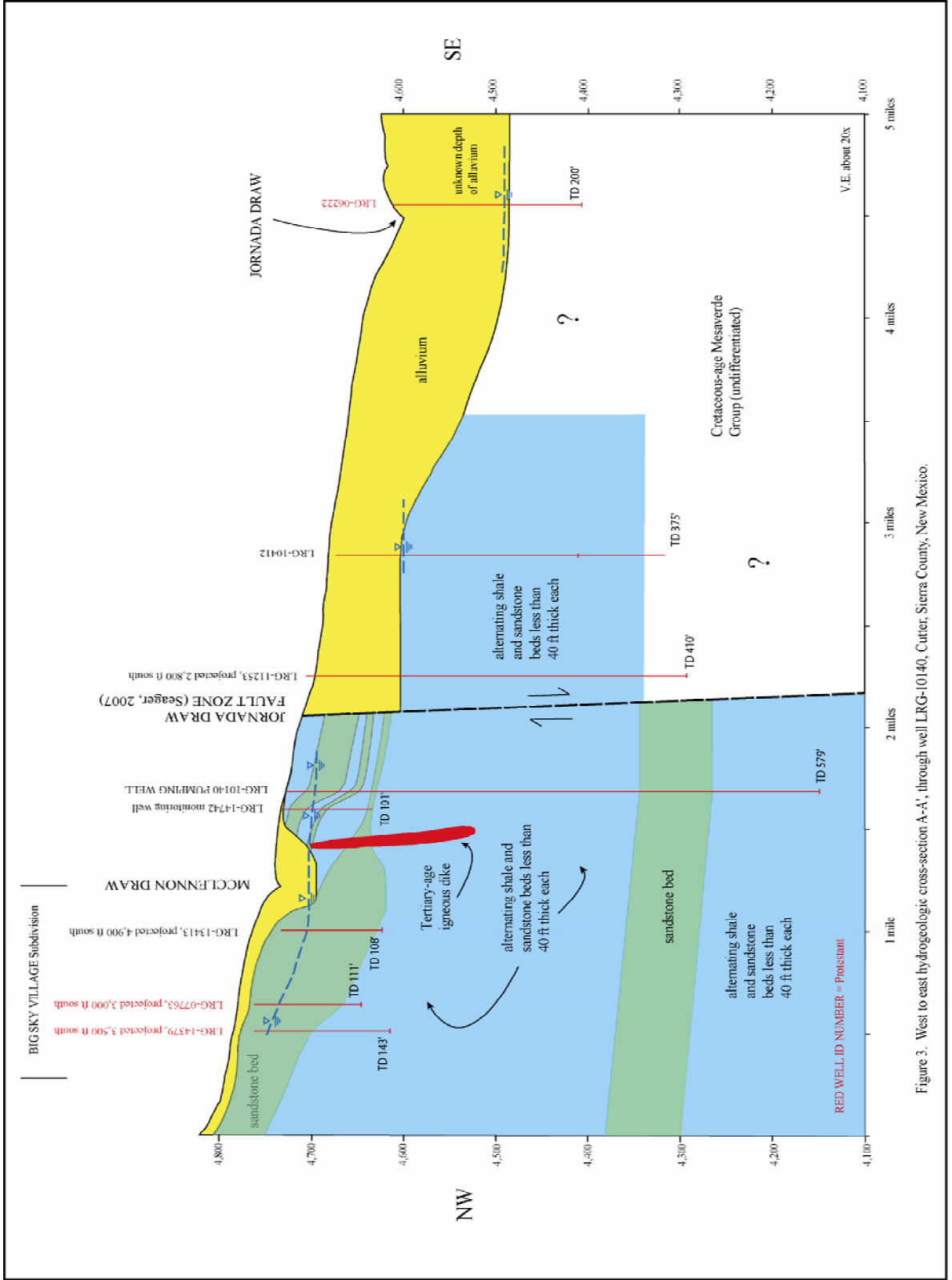


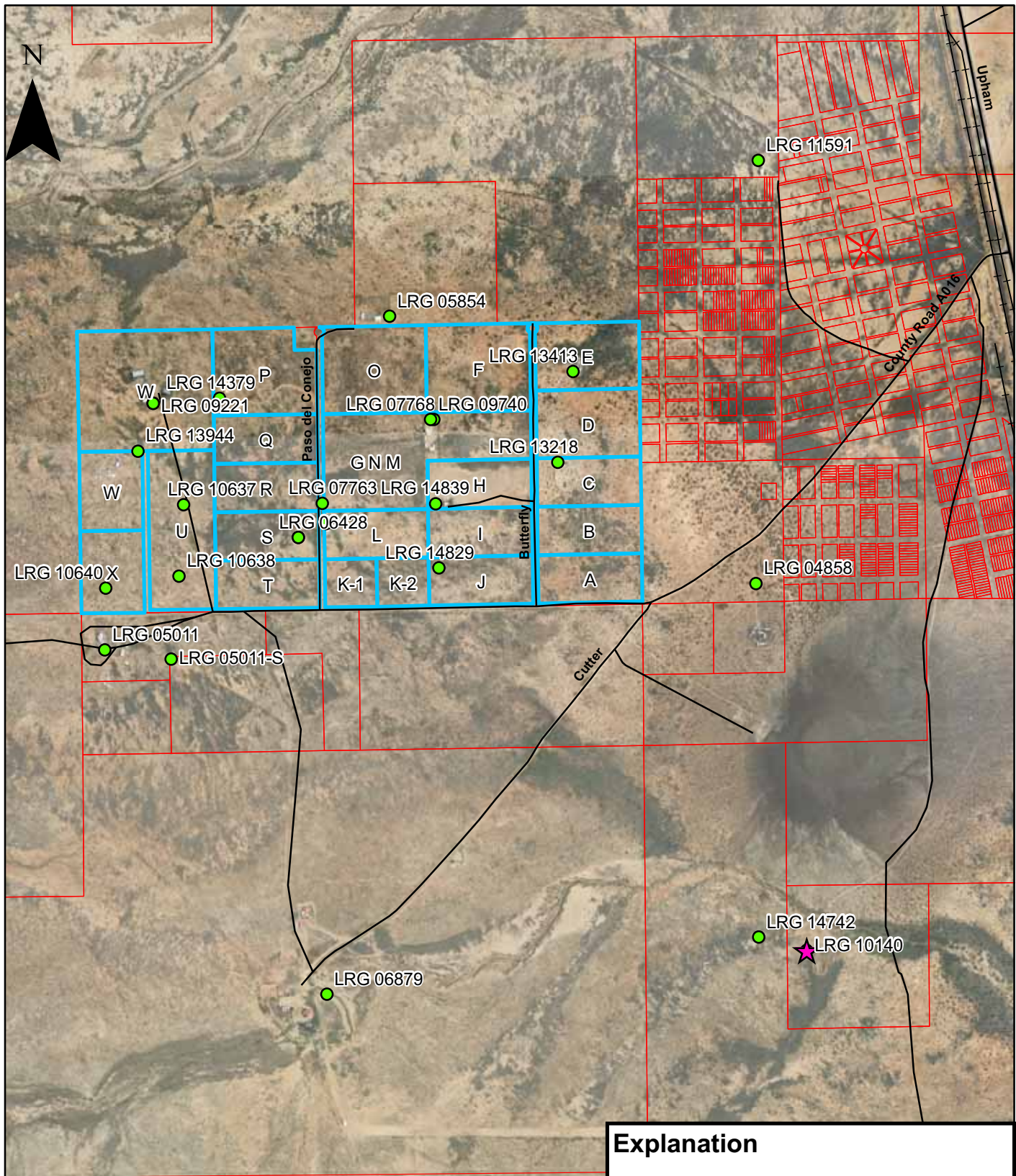
Figure 3. West to east hydrogeologic cross-section A-A', through well LRG-10140, Cutter, Sierra County, New Mexico.

Background

Cutter History

Cutter was founded in the early 1900s as a train stop on the Santa Fe Railroad running between El Paso and Albuquerque. The name Cutter is reported to have originated from the name of a Santa Fe railroad engineer. Mining of Vanadium in the nearby Caballo Mountains stimulated development in the area, as the ore was loaded in town and shipped out on the railway. It is estimated that, at its peak, the town of Cutter had a population of around 3,500 people (SCHS, 1979). The Victoria Chief Mining Company constructed a mill in Cutter to prepare the ore for shipment and employed about 300 men to run the operation. Several hundred other men were employed in the mines (Mitchell & Tooley, 1973). The mining boom did not last long for the area and when the mines closed in the early 1930s, most residents left. A few businesses remained for a short time afterwards, but would soon close down as well. The few that remained consisted of ranches and a Santa Fe depot which was used by the railroad to house employees. Once the Santa Fe changed over to diesel locomotives in the late 1940s, the stop was no longer necessary and Cutter was nearly completely abandoned.

Today, Cutter is an unincorporated area of subdivided land with approximately 40 residents. The Big Sky Village Subdivision, which was created in the early 1980s, makes up the majority of developed land and can be seen in Figure 4 (pg. 10). There are 42 wells that are identified in the NMOSE records in the Cutter area (Table 1). A map of these well locations is presented in Figure 5 (pg. 11).



Explanation

- ★ Focus Well
- Well
- Road
- +—+— Railroad
- Big Sky Village Subdivision
- Cutter Parcels

Figure 4. Map of the Big Sky Village Subdivision Cutter, NM

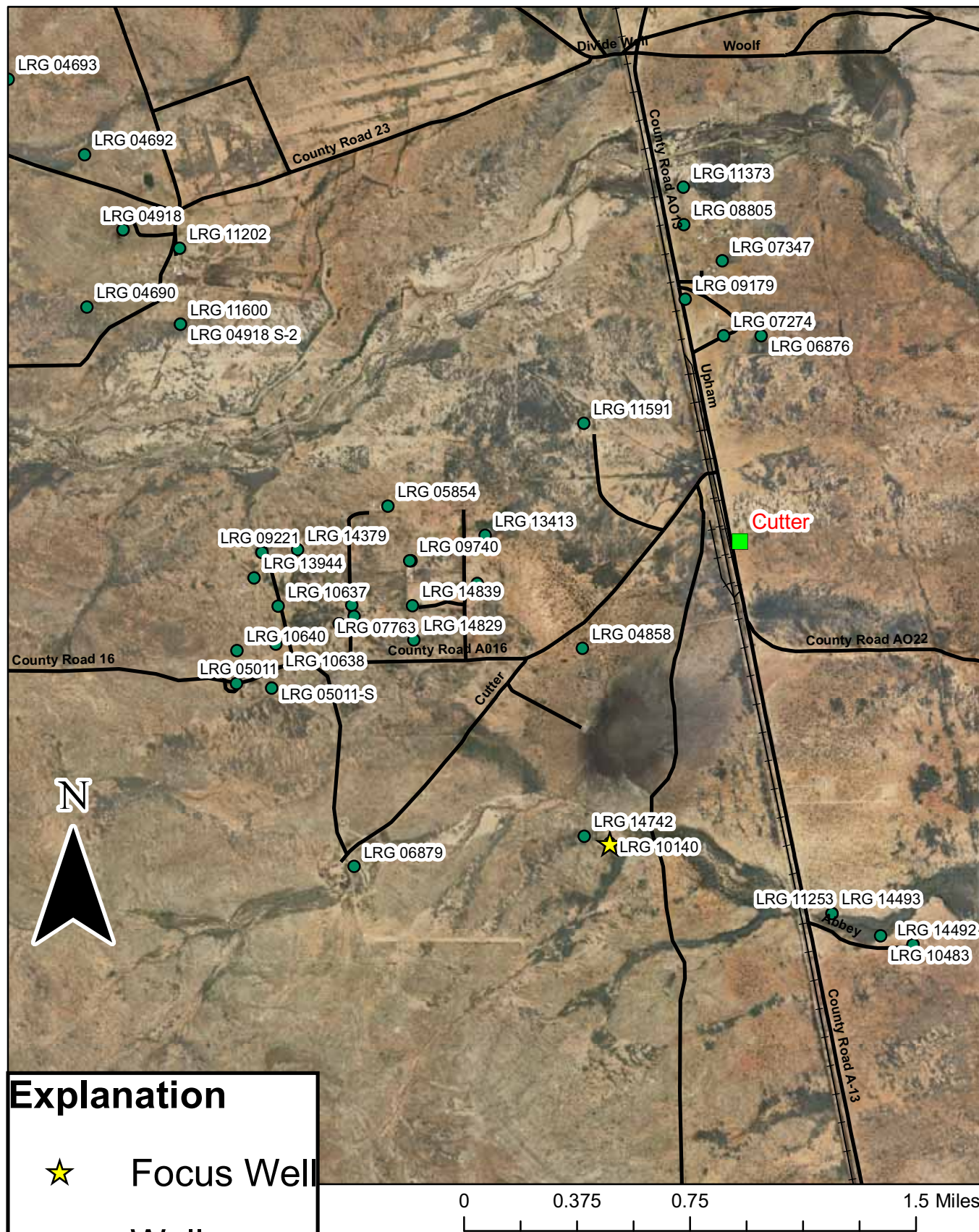


Figure 5. Map showing locations of wells in Cutter, NM area

Table 1. Permitted wells in Cutter and surrounding area (NMOSE, 2011).

WR File Nbr	Permitted Use	Permitted Diversion, ac-ft/yr	Owner	POD ¹ Number	X, UTM83 Z13N, Meters	Y, UTM83 Z13N, Meters
LRG 04690	IRR	TBD	RIO GRANDE ALBQ & ACADEMY CORP	LRG 04690	307780	3661132
LRG 04692	IRR	TBD	GUARANTEED EQUITIES, INC.	LRG 04692	307789	3661945
LRG 04693	IRR	TBD	RIO GRANDE ALBQ & ACADEMY CORP	LRG 04693	307392	3662359
LRG 04858		15.5	CHARLES D GOETZ (CUTTER CATTLE CO.)	LRG 04858	310377	3659239
LRG 04918	IRR	TBD	BELL GROUP INC	LRG 04918	307985	3661538
				LRG 04918 S	307985	3661538
LRG 04918 S-2			BELL GROUP INC	LRG 04918 S-2	308278	3661027
LRG 05011	IRR	TBD	HAROLD & EUNICE D. MCCARTY	LRG 05011	308524	3659100
LRG 05011-S			HAROLD & EUNICE D. MCCARTY (ADDIE'S WELL)	LRG 05011 S	308711	3659069
LRG 05854	DOM	3	GARY DICKSON (CHRIST)	LRG 05854	309358	3660025
LRG 06243	DOM	3	ROD HILLE	LRG 06243	305333	3659179
LRG 06428	DOM	3	SHERRY LYNN EUNGARD	LRG 06428	309082	3659404
				LRG 06428 D-REP	309082	3659404
LRG 06876	STK	10	HOWE RANCH	LRG 06876	311376	3660880
LRG 06879	DOM	6	BARBARA BROWN	LRG 06879	309127	3658106
LRG 07274	STK	3	WALTER AND WANDA LEWIS	LRG 07274	311176	3660887
LRG 07347	STK	6	RUTH V BALDWIN	LRG 07347	311179	3661286
LRG 07763	IRR	TBD	H WAYNE JOHNSON	LRG 07763	309152	3659500
LRG 07763 S	IRR	TBD	H WAYNE JOHNSON	LRG 07763 S	309161	3659442
LRG 07768	IRR	TBD	JULIE MUNOZ	LRG 07768	309475	3659729
LRG 08805	DOM	3	WILLIAM W MCCORD	LRG 08805	310979	3661486
LRG 09179	DOM	3	LARRY G LEWMAN	LRG 09179	310976	3661087
LRG 09221	DOM	3	ARCHIE BAILEY	LRG 09221	308680	3659796
				LRG 09221 REP	308680	3659796
LRG 09740	DOM	3	GWYN E THURMAN	LRG 09740	309466	3659729

LRG 10140	COM	394.85	TOBY ROMERO	LRG 10140	310493	3658194
LRG 10412	DOM	3	LINDA ALLEN	LRG 10412	312099	3657609
LRG 10483	EXP	0	GREGORY A. ALLEN	LRG 10483	311926	3657659
LRG 10637	PMT	0	BOBBY R. & DAPHNE TRIBBLE	LRG 10637	308759	3659506
LRG 10638	PMT		BOBBY R. & DAPHNE TRIBBLE	LRG 10638	308740	3659304
LRG 10640	DOM	3	THOMAS WELLMAN	LRG 10640	308531	3659276
LRG 11202	DOM	3	RODERICK G HILLE	LRG 11202	308284	3661433
LRG 11253	DOM	3	DAVID C KUNZ	LRG 11253	311671	3657786
LRG 11373	DOM	3	LAWRENCE OTT	LRG 11373	310982	3661685
LRG 11591	DOM	3	JOE TURNER TRUST	LRG 11591	310416	3660439
LRG 11600	DOM	3	RODERICK G. HILLE	LRG 04918 S-2	308278	3661027
LRG 13218	DOM	1	QUANTUM LIZARD LLC (HOROSCHAK)	LRG 13218	309824	3659598
LRG 13413	DOM	3	DARLENE A GLADWELL ESTATE (WILLAMS)	LRG 13413 POD1	309873	3659855
LRG 13944	PDM	3	KIM TRIBBLE	LRG 13944 POD1	308633	3659662
LRG 14379	PDL	3	SANDRA TATE	LRG 14379 POD1	308869	3659805
LRG 14682	PDL	3	RODERICK G HILLE	LRG 14682 POD1	305979	3661983
LRG 14742	EXPL	0	TOBY ROMERO (OBSERVATION WELL)	LRG 14742 POD1	310358	3658235
LRG 14829	DOM	1	WILLIAM D. CRISP	LRG 14829	309478	3659307
LRG 14839	DOM	1	JAMES E. SMITH	LRG 14839 POD1	309474	3659490

¹ Point of Diversion

Romero Well History

Well LRG-10140 is the production well that is the subject of much of the analysis in this report. LRG-10140's early history is much like Cutter's history in that it was dependent upon the railroad. The well was drilled in 1921 by A.A. Riggs to supply water for locomotives. Known at the time as Well No.6, this steam powered system was reported to originally pump around 150 gallons per minute (gpm). A log of the well is attached in the appendices along with a diagram of the original pumping apparatus and train schedule that was used to estimate pumping rates from the well. The well was drilled to a total depth of 578 ft bgl and left as an open borehole. The well was pumped from 1921 to 1966 for railroad use and then again from 1992 to 1995 for livestock watering purposes. The present owner claims a diversionary amount of 394.85 ac-ft/yr based upon a calculation from the train schedules.

Mine production was used to estimate water use by the railroad. The maximum tonnage of ore from mining was 4,541,000 tons in 1944. The gross tons divided by 600 tons per train equals 630 trains per month. 630 trains per month multiplied by 17,000 gallons per train equals 10,721,805 gallons per month which is equivalent to 32.9 ac-ft per month or 394.85 ac-ft/yr (NMOSE, 2011). This permitted amount only applies to the well's original use for the railroad. This amount is not permitted for any other purposes.

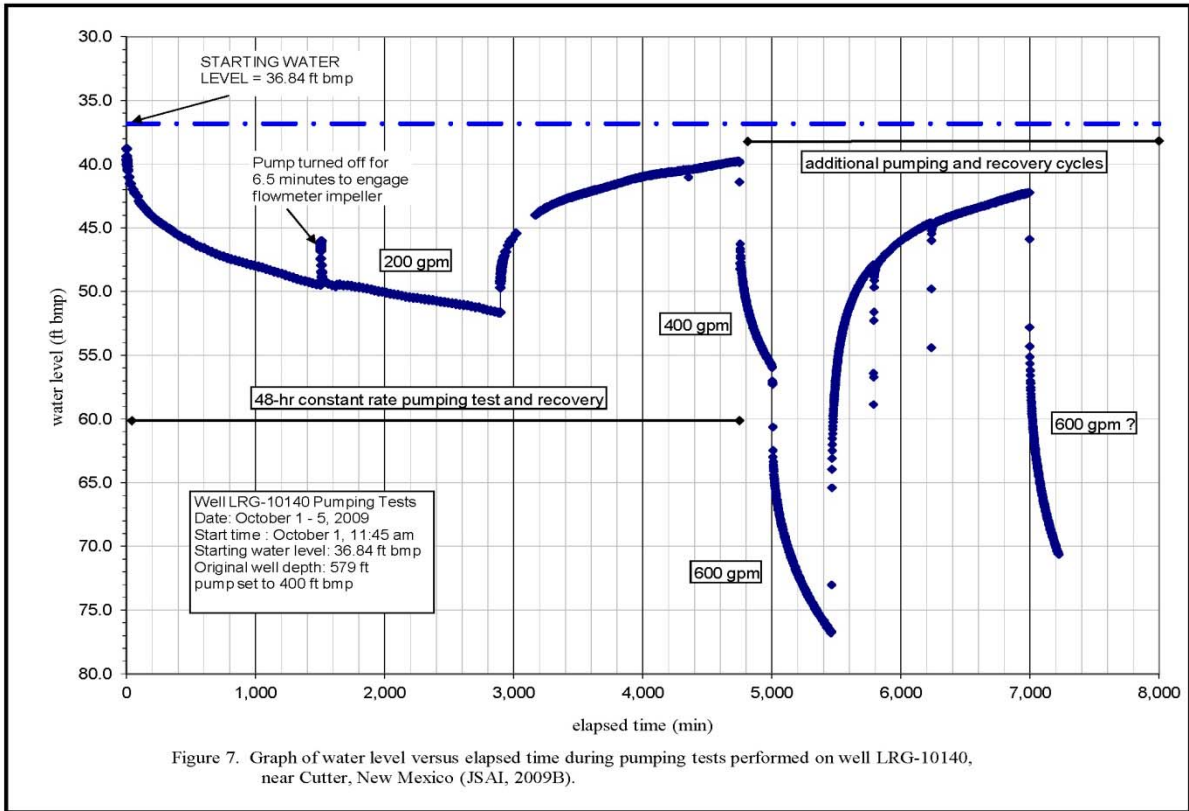
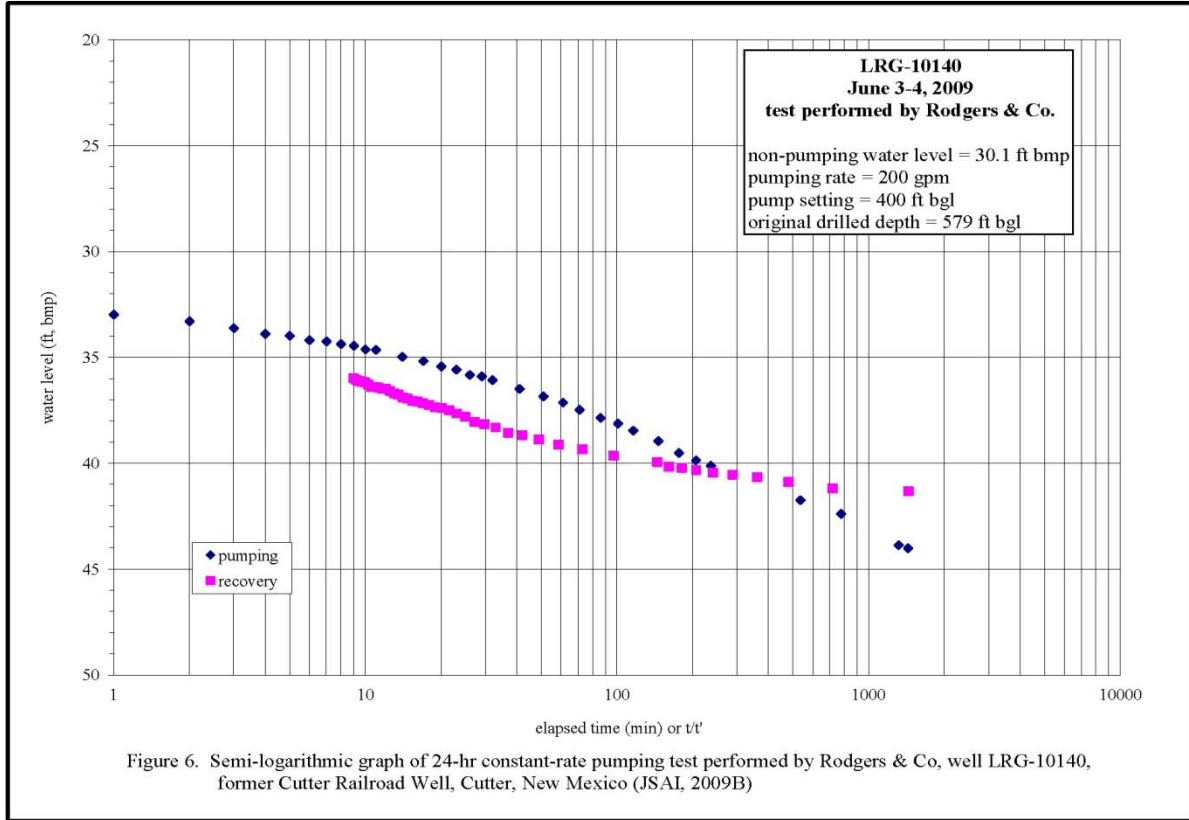
Today, the steam powered pump is on display at a nearby museum in Truth or Consequences, New Mexico.

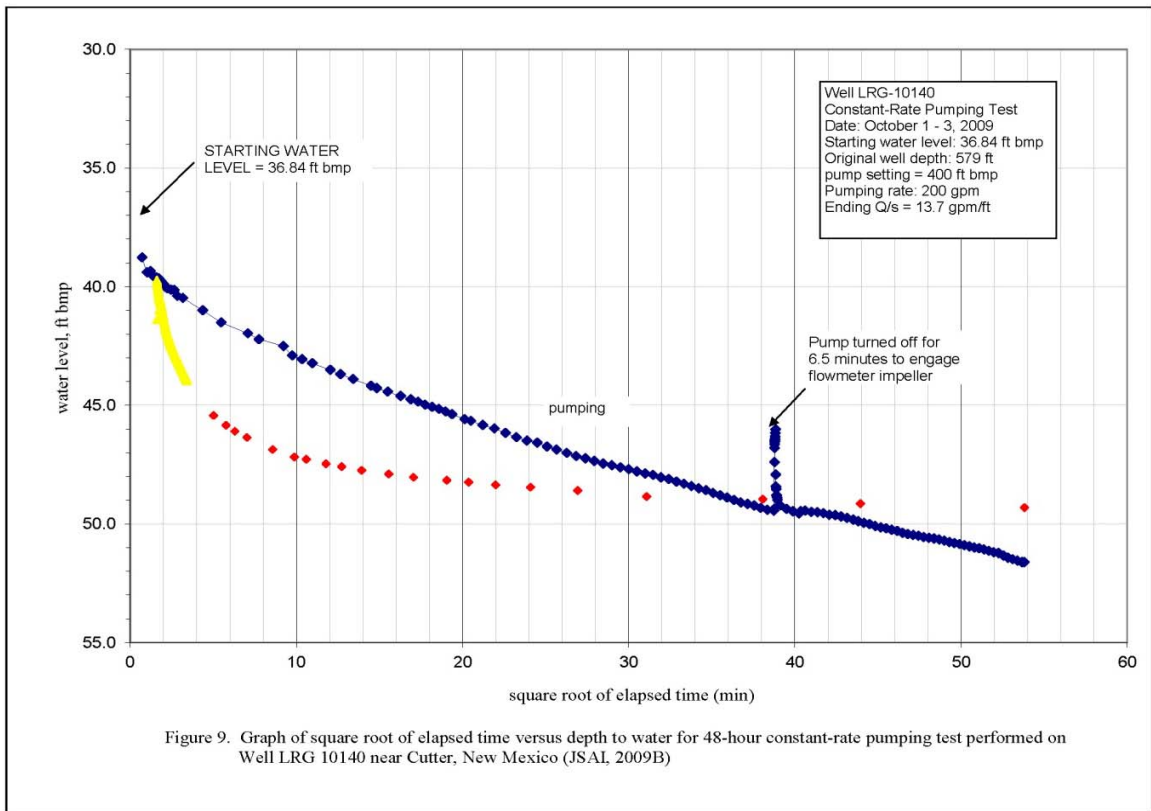
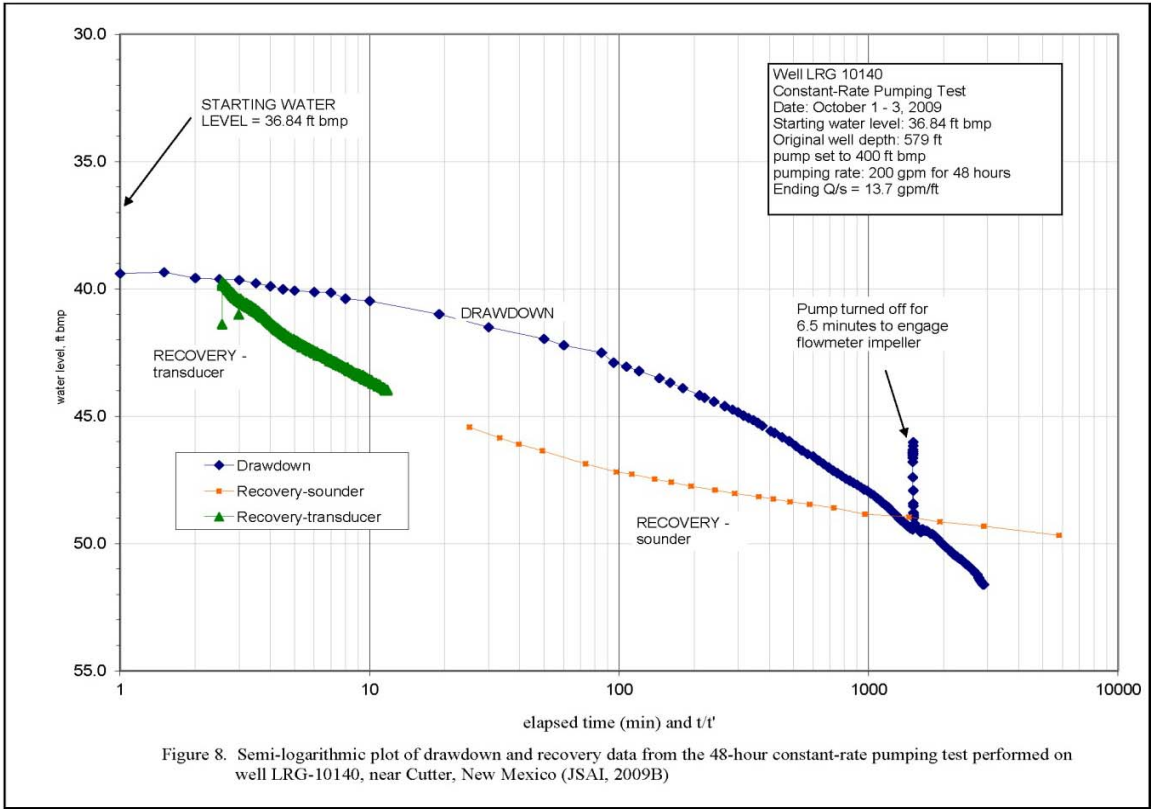
When construction began for Spaceport, a temporary water source was needed while issues associated with water rights applications for the facility were resolved. LRG-

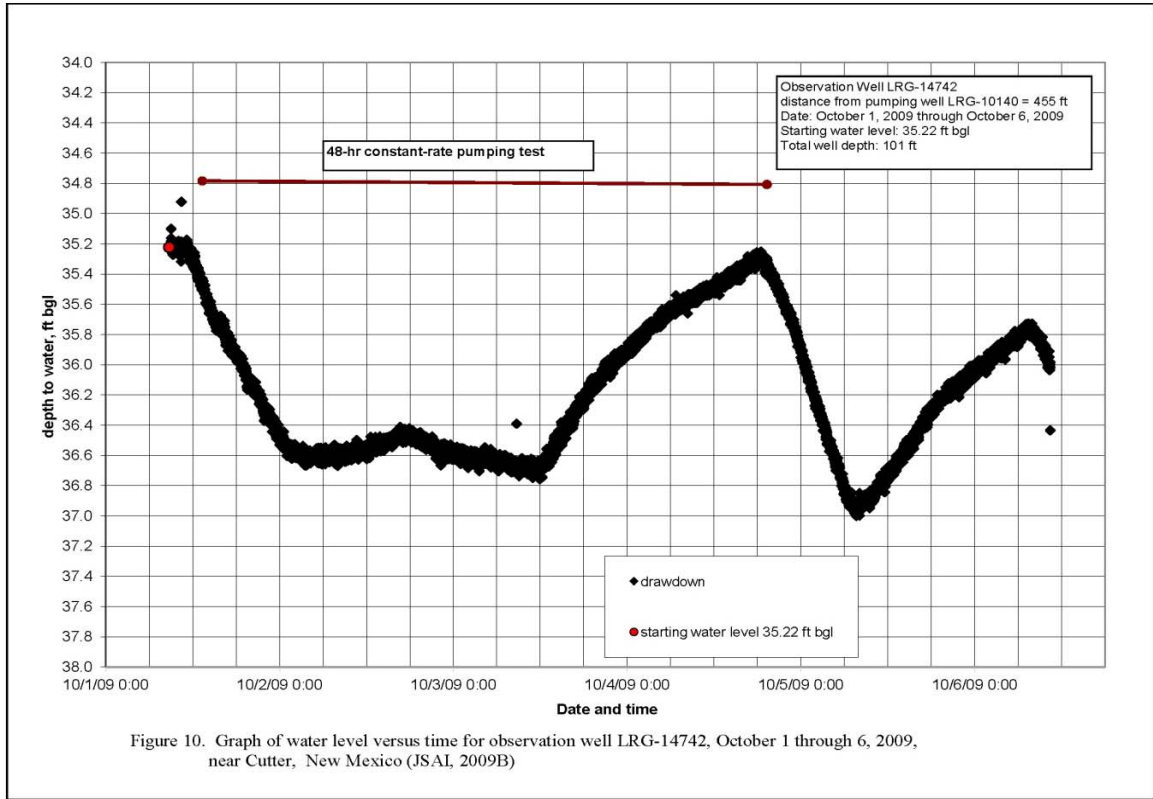
10140 was restored to service by FNF Construction (FNF) of Albuquerque, New Mexico. Before the well was put back into production, existing water rights from other wells were transferred to LRG-10140 and a pumping test was performed and evaluated by John Shomaker & Associates, Inc. (JSAI). Between October 1 and 3, 2009, a total of 583,300 gallons were pumped during a 48-hour constant-rate pumping test (JSAI, 2009B). The well was pumped at an average rate of 200 gpm during the test. Water levels in two monitoring wells were measured, one new well constructed for this test and one existing well, to assist in the evaluation. The new monitoring well (LRG-14742) was cased to a depth of 101 ft bgl and screen was set from 41-101 ft bgl. The following conclusions were made by JSAI (JSAI, 2009B).

- The shapes of the drawdown and recovery curves on Figures 6 through 9 (pgs. 17-18) indicate a bounded fractured aquifer of limited size. The limits of the fractured aquifer are best explained by the geologic boundaries surrounding LRG-10140 (Fig. 2, pg. 7), including the Jornada Draw Fault to the east, a series of dikes to the south and west, and the Black Hill intrusion to the north.
- Due to the aquifer characteristics, a traditional Theis analysis of the pumping test data is not valid.
- The observed drawdown and recovery demonstrates that the observation wells are hydraulically connected to the pumping well LRG-10140.
- The fractured system tapped by LRG-10140 appears to recharge and recover from pumping, as indicated by projecting late-time recovery shown on Figures 7 through 10 (pgs 17-19). The bounded, fractured aquifer may be connected to the area northwest of LRG-10140 between mapped dikes and Black Hill.

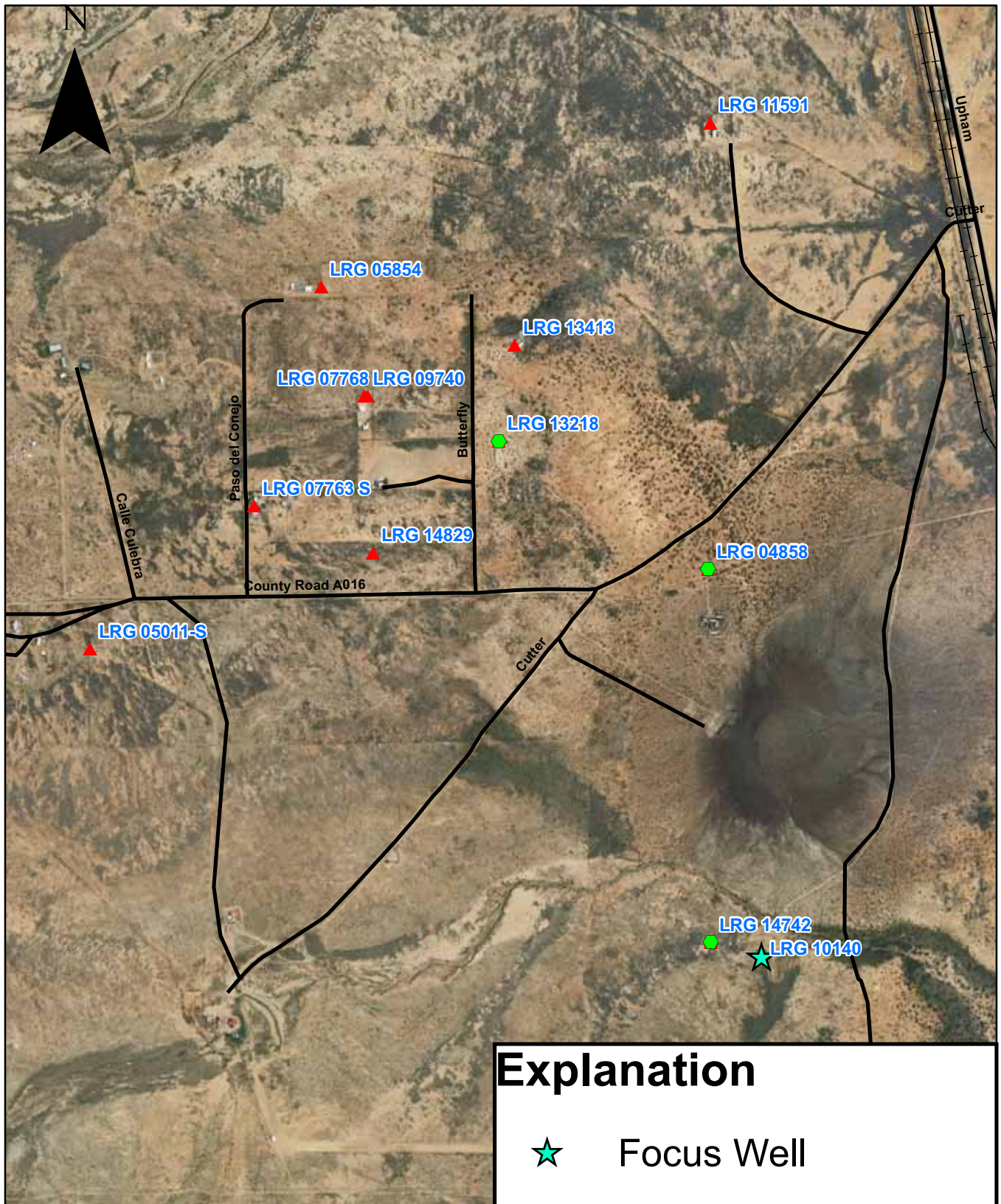
- At a pumping level of 350 ft below ground level (bgl), approximately 50 percent of the sandstone units would be dewatered around the well, and the yield would begin to decline significantly. Therefore, the maximum recommended pumping level is 350 ft bgl. Based on the analysis of the pumping data, continuous pumping at 200 gpm for five years would result in a pumping level of approximately 350 ft bgl. This equates to 320 ac-ft/yr for five years, or a total of approximately 1,600 ac-ft/yr in five years.
- Recharge to the localized aquifer supplying LRG-10140 is estimated at 50 gpm (80 ac-ft/yr) from water-level recovery trends. Therefore, pumping 1,600 ac-ft from LRG-10140 would take approximately 20 years to fully recover.
- All wells of other ownership are outside of the bounded fractured aquifer tapped by LRG-10140 (Figs. 2 and 5). Therefore, drawdown impacts propagating to neighboring wells is unlikely. There is a remote possibility the drawdown caused by LRG-10140 would propagate to the northwest.
- Using the test data and geologic relationships between LRG-10140 and LRG-14742 (monitoring well), it is estimated that the drawdown in the shallow sandstone units will not propagate much beyond 1,000 ft from LRG-10140 regardless of pumping rate or duration.
- There are no known wells of other ownership located 1,000 ft northwest of LRG-10140 (Fig. 5, pg.11). The closest known well of other ownership is LRG-4858, which is located approximately 3,500 ft northwest of LRG-10140 (Fig 5). The analysis results in no significant hydrologic impacts on known wells of other ownership (JSAI, 2009B).









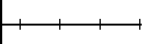


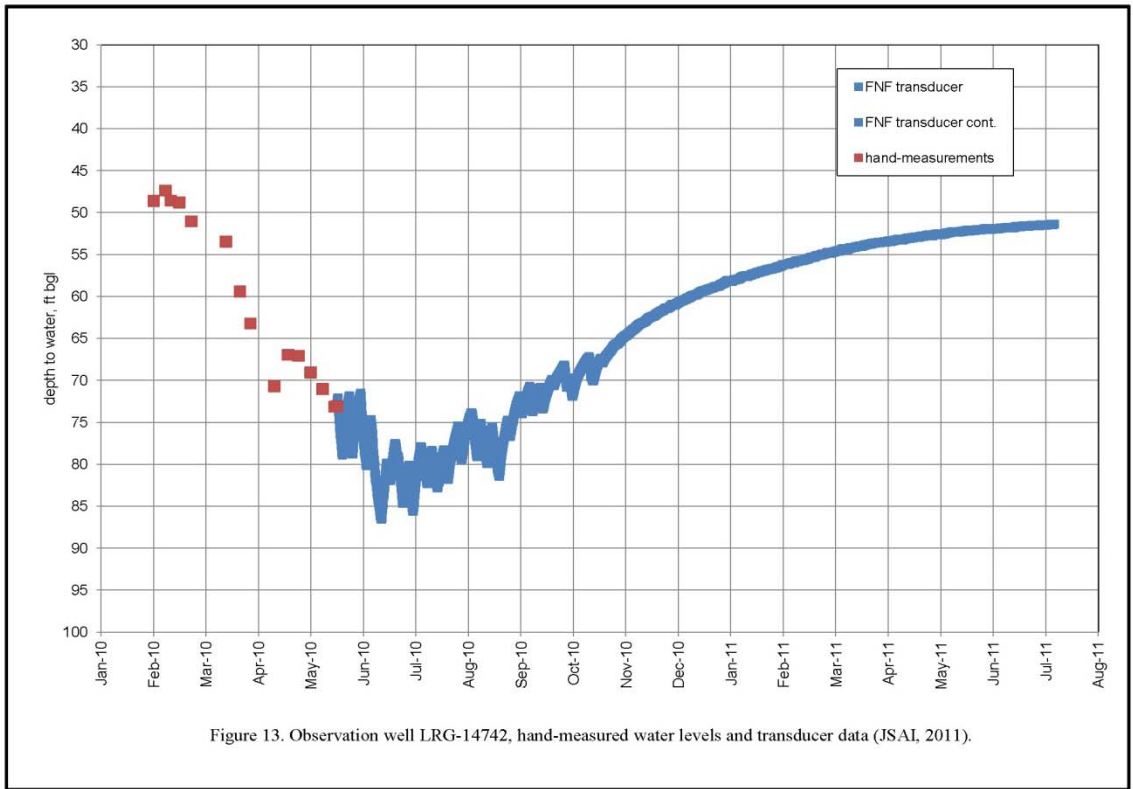
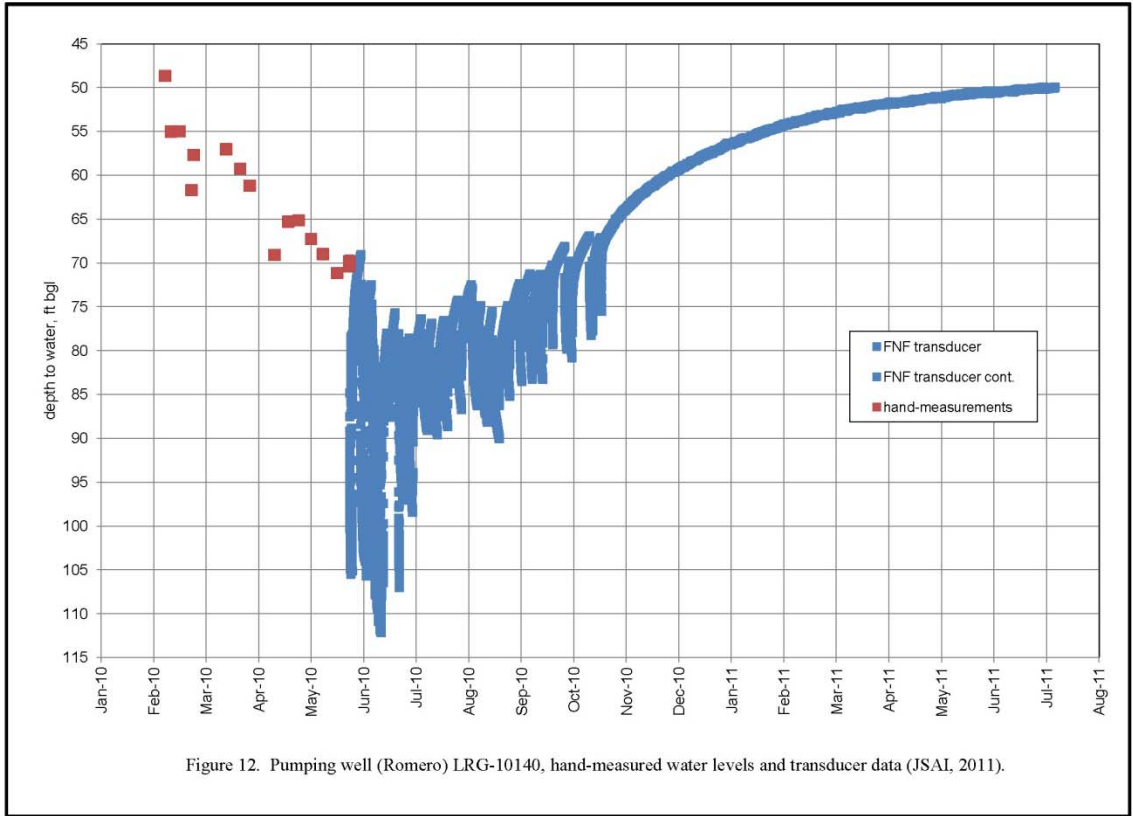
After testing the well, it was put into service to provide water for construction at the Spaceport. Between September 2009, and June 2010, a total of 138 ac-ft were pumped (Finch, 2010). Around April 2010, residents in Cutter began to complain to the NMOSE about drawdown in domestic wells and wells gone dry. As a result, transducers were installed by FNF in LRG-10140, LRG-14742 (Observation Well), LRG-4858 (Cutter Well), and LRG-13218 (Horoschak). In addition to these wells, the NMOSE took hand measurements from LRG-5011-S (Addie's), LRG-13413 (Williams), LRG-5854 (Christ), LRG-7768 (Munoz), LRG-7763-S (Johnson), LRG-11591 (Turner Trust), LRG-14829 (Crisp), and LRG-9740 (Thurman), from May 2010, through March 2011. A map of the mentioned wells can be found in Figure 11 (pg. 20). The transducer data, along with the NMOSE measurements are available in Figures 12 through 16.



0 0.125 0.25 0.5 Miles

Figure 11. Map showing locations of wells with water level data, Cutter, NM

Explanation	
	Focus Well
	Transducer Wells
	Hand-measured Wells
	Road
	Railroad



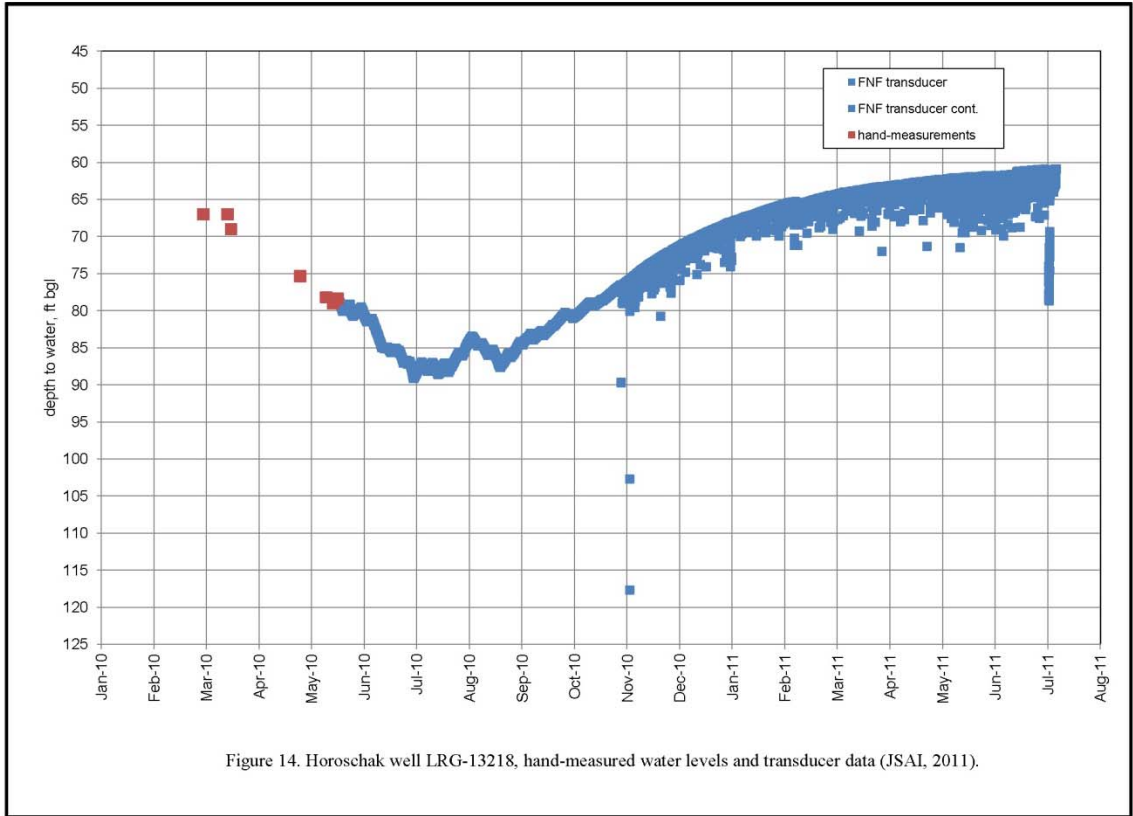


Figure 14. Horoschak well LRG-13218, hand-measured water levels and transducer data (JSAI, 2011).

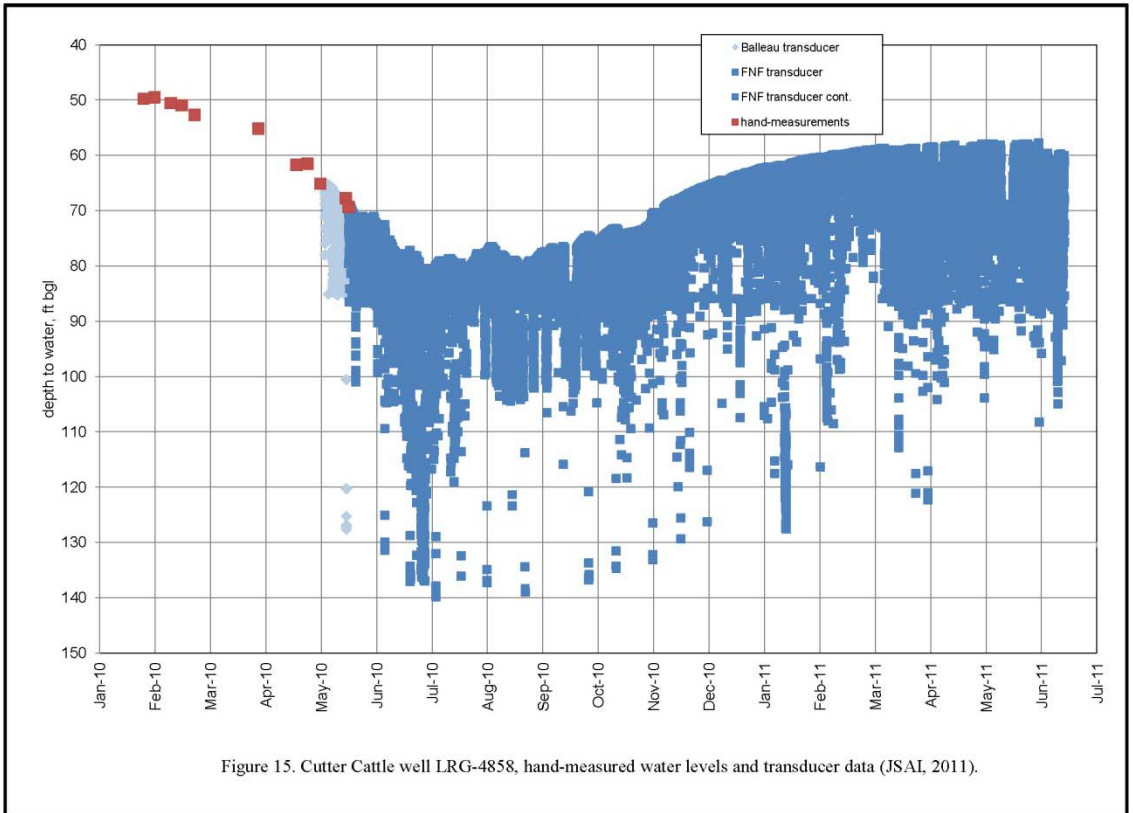
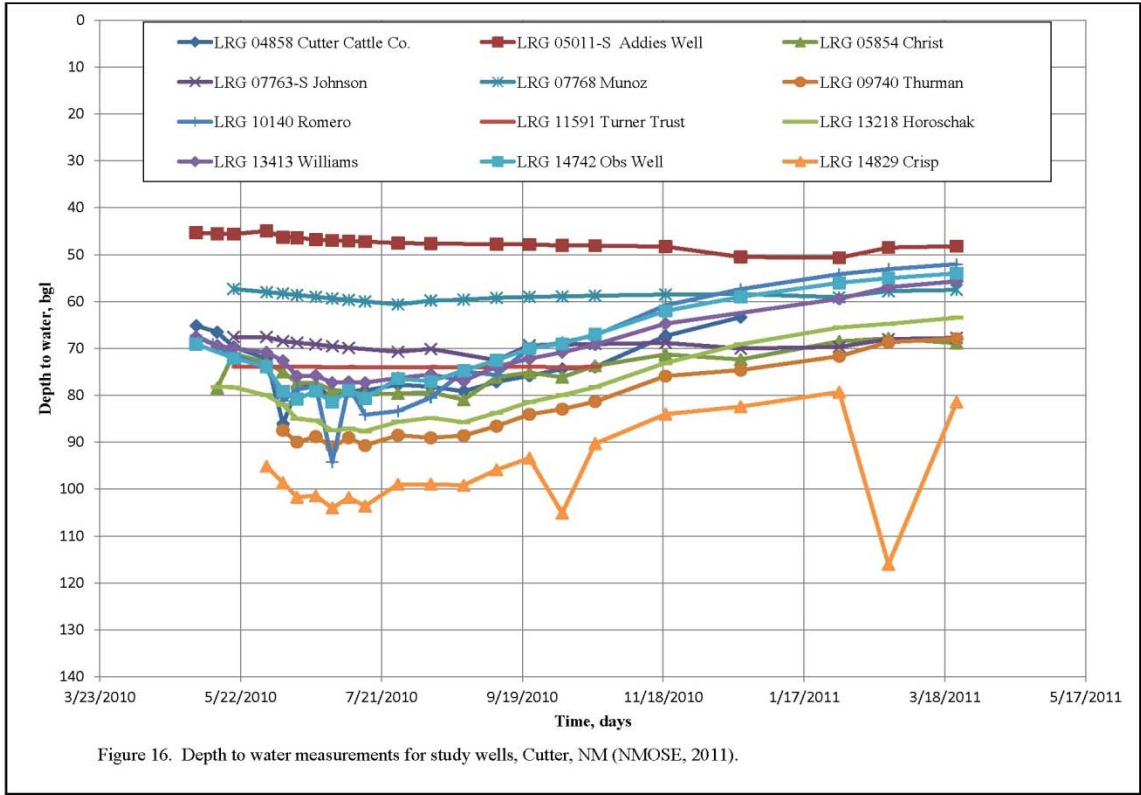


Figure 15. Cutter Cattle well LRG-4858, hand-measured water levels and transducer data (JSAI, 2011).



Original estimates of drawdown did not anticipate impacts that were observed in and around Cutter. As mentioned by JSAI (2009B), any possible drawdown impacts would propagate to the northwest but should not extend much beyond 1,000 ft. Drawdown impacts were noted at much greater distances meaning, the storage held within the system was much less than originally estimated. Areas of high transmissivity, due to fractures, were also apparent. This groundwater hydrology was proving to be more complex than originally expected.

Projected Water Demand

Understanding the geologic conditions of this aquifer system is one key element to determining the impacts of future development. Another key factor, is estimating the

amount of water that is needed to support the community. While current literature contains various amounts of per capita water use on a daily basis (gpcd), limited studies have been conducted to estimate what residents with private wells, in arid rural settings, consume. Many believe that rural residents use greater amounts of water on a daily basis than residents in urban areas, while others argue those outside the city limits use less. For the purposes of this study, estimations based upon the following references were used.

The amount of water that can be pumped from a domestic well in New Mexico is only subject to limitations in areas considered “declared underground basins” by the State Engineer. After declaration, domestic wells, issued permits under § 72-12-1 (NMAC), were first limited to 3 ac-ft/yr (N.M.S.A. 1978 §§ 72-12-1.1 - 72-12-1.3, 2003). In 2006, the amount was reduced to 1 ac-ft/yr (N.M.A.C. 19.27.5). No public notice is required or protest allowed on domestic well applications. Domestic well applications may be approved, rejected, or approved with conditions by the NMOSE. Conditions may be imposed on a permit, such as; minimum distance from adjacent wells, metering and monitoring requirements, compliance with local ordinances, restrictions on purpose of use, or other conditions as the situation warrants. A permit may be cancelled if a permit holder fails to comply with conditions (Utton Center, 2009).

In 2000, the Hydrology Bureau of the NMOSE prepared a paper in response to a legislative initiative which estimated statewide average domestic-well diversions at 0.35 ac-ft/yr per residence, based on a demand of 114 gpcd and an average of 2.74 persons per residence (NMOSE, 2000). This paper was then updated with information from Wilson et al. (2003), which represented somewhat smaller per-capita use. For the purpose of estimating withdrawals for the self-supplied domestic population, in most counties an

areawide average of 80 gpcd is used. In counties where water requirements for landscape irrigation and evaporative cooling are more prevalent, an areawide average of 100 gpcd is used (Wilson et al., 2003). In many instances, one domestic well serves several households. The Wilson report is designed as one well per household and, because of this, can be an underestimation of the actual amounts pumped.

The United States Geological Survey (USGS) has reported on self-supplied domestic water use. Table 2 is a compilation of data taken from the USGS on a state-by-state basis. The self-supplied domestic water use for the year 2000, and the population dependent upon it, is given for each of the western interior states. These estimates are generally taken from data gathered by state agencies, and for that reason probably reflect the assumptions of the state agencies rather than rigorous study by USGS (JSAI, 2009A).

Table 2. Estimates of self-supplied domestic diversion, by state, for year 2000, from USGS

State	Population supplied by domestic wells	Diversion, ac-ft/yr	Acre-feet per year per capita	Estimated annual withdrawal, gallons per capita per day
New Mexico	360,000	35,200	0.098	87.2
Arizona	265,000	32,400	0.122	108.8
Colorado	555,000	74,900	0.135	120.4
Idaho	366,000	95,600	0.261	232.8
Kansas	193,000	24,200	0.125	111.5
Montana	238,000	19,400	0.082	72.7
Nebraska	324,000	54,300	0.168	149.9
Nevada	124,000	25,200	0.203	181.1
Oklahoma	299,000	28,500	0.095	84.8
Texas	1,190,000	147,000	0.124	110.6
Utah	56,200	18,000	0.320	285.5
Wyoming	87,500	7,360	0.084	74.9
Entire U.S.	43,500,000	4,026,100	0.093	82.6

(JSAI, 2009A)

Data provided from metering related to FNF's pumping of LRG-10140 gives an actual amount for a resident in Cutter using a domestic well. Mr. Smith, owner of LRG-14839, had a meter placed on his domestic well. From February to June of 2011, Mr. Smith average daily use was 420 gallons for two residents in his household (210 gpcd). This provided a true value for water use and was used for calculations discussed later in the paper.

Methodology

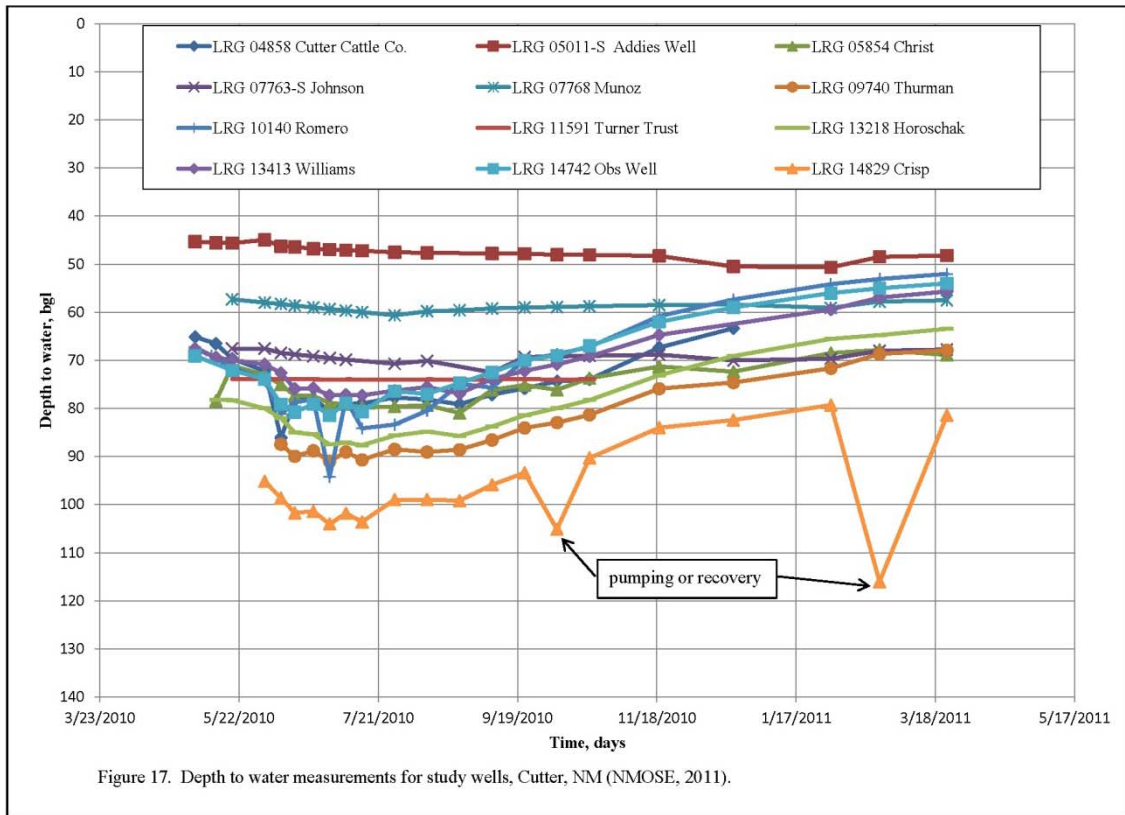
Water Level Data

Much of the data analyzed in this project was from monitoring related to use of LRG-10140 and was provided by JSAI and the NMOSE. Transducers installed by FNF on wells LRG-10140, LRG-14742, LRG-4858, and LRG-13218, along with water levels collected by the NMOSE at wells LRG-5011-S, LRG-13413, LRG-5854, LRG-7768, LRG-7763, LRG-11591, LRG-14829, and LRG-9740, provided information that was used to determine the extent of the aquifer tapped by the Romero Well. Table 3, below, lists the wells for which groundwater measurements were recorded and Figure 11 (pg. 20), shows the locations of these wells.

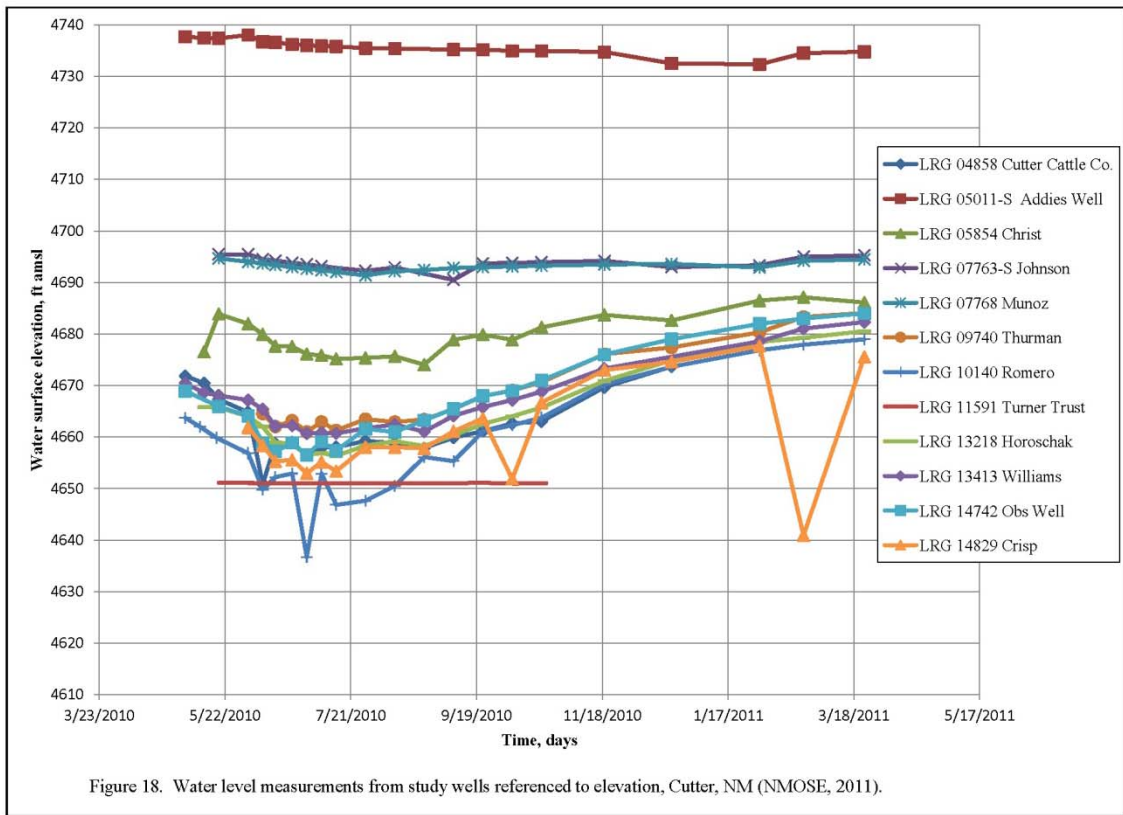
Table 3. Wells with water level measurements.

WR File Nbr	Use	Diversion, ac-ft/yr	Well Name	X, UTM 83, Z13N meters	Y, UTM 83, Z13N meters	Well Depth, ft bgl	Surface Elevation, ft amsl
LRG 04858		15.5	CUTTER CATTLE CO.	310377	3659239	164	4737
LRG 05011-S	IRR	TBD	ADDIE'S WELL	308711	3659069	180	4783
LRG 05854	DOM	3	CHRIST	309358	3660025	190	4755
LRG 07763 S	IRR	TBD	JOHNSON	309161	3659442	111	4763
LRG 07768	IRR	3	MUNOZ	309475	3659729	152	4752
LRG 09740	DOM	3	THURMAN	309466	3659729	200	4752
LRG 10140	COM	394.85	ROMERO WELL	310493	3658194	578	4731
LRG 11591	DOM	3	TURNER TRUST	310416	3660439	170	4725
LRG 13218	DOM	1	HOROSCHAK	309824	3659598	135	4744
LRG 13413	DOM	3	WILLIAMS	309873	3659855	108	4738
LRG 14742	EXPL	0	OBSERVATION WELL	310358	3658235	100	4738
LRG 14829	DOM	1	CRISP	309478	3659307	240	4757

The 4 wells with transducers began logging water levels in May of 2010, and were programmed to take readings every 15 minutes. The NMOSE measurements also began in May of 2010, and were taken manually. These wells were measured at varying intervals. To compare the data, the dates on which NMOSE measurements were taken were used and compared to the transducer data on that corresponding day. Water levels from the transducer wells were selected by the highest, or most shallow, reading from that selected date. This was done to ensure that a pumping water level was not selected from the data set. It should be noted that some of the NMOSE hand measurements may be pumping water levels, or somewhere in the stage of well recovery. This is evident in Figure 17 with regard to the water level reading from LRG-14829 on October 2, 2010, and again on February 22, 2011.



Once the water level readings were organized, surface elevations at well heads were referenced from satellite imagery using “Google Earth”. Surface elevations were recorded as feet above mean sea level (famsl). Figure 18 shows the results of organizing the water level data with relation to surface elevation.



Romero Well Pumping Data

The amount of water pumped from LRG-10140 was taken from meter readings provided on the NMOSE Water Administration Technical Engineering Resource System (WATERS) database. Meter readings began on June 30, 2009, and continued through November 2, 2010. Figure 19 is a graph of metered usage and Table 4 summarizes the amount pumped by calendar year.

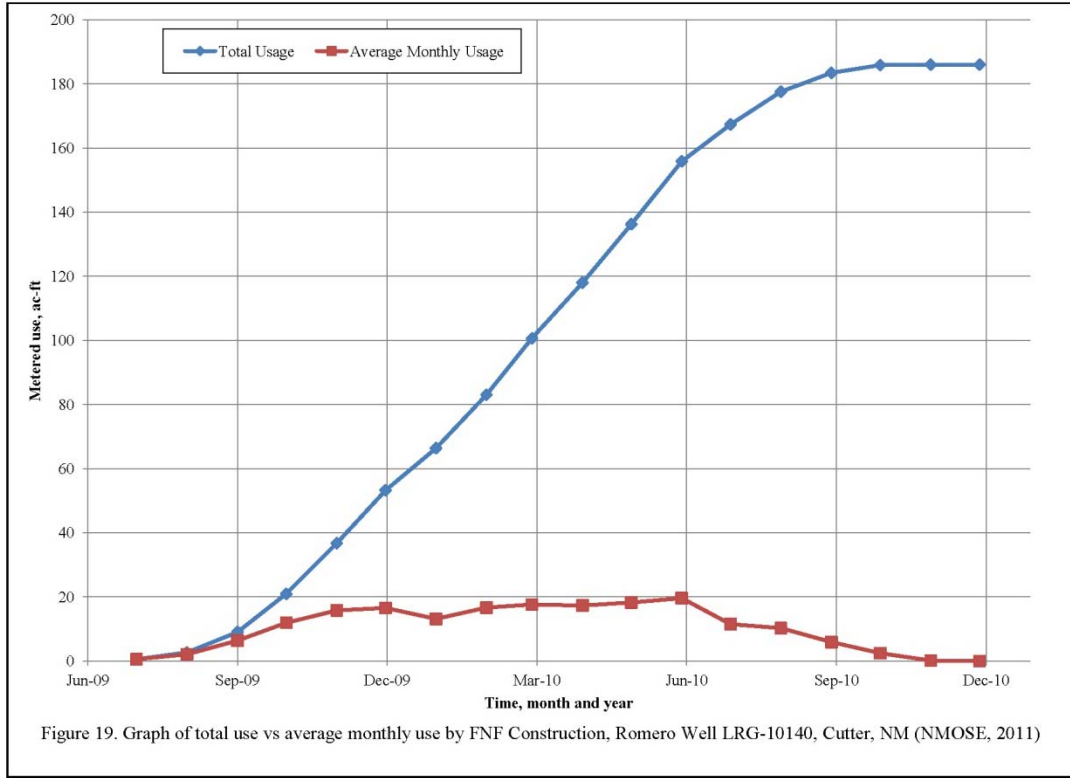


Table 4. Summary of water pumped from LRG-10140 by calendar year, NMOSE meter readings.

Calendar year	Amount pumped, ac-ft
2009	53.244
2010	132.753
Total	185.997

Current and Future Demand for Water

While the recent pumping from LRG-10140 far exceeded the usual demand for water in the Cutter area, it did give an indication of what future development could do to the aquifer. One large well pumping at a rate of 200 gpm is the same as 20 small wells pumping at a rate of 10 gpm.

For the purposes of this study, a value of 16.5 ac-ft/yr was given to LRG-07763-S designated TBD based on its use as an irrigation well. The NMOSE record shows an irrigated amount of 5.5 acres of land. 5.5 acres of land multiplied by 3 ac-ft/acre equals 16.5 ac-ft/yr. Since only a certain number of wells are assumed to be within the designated zone mentioned previously, the following information was used to estimate current and future demand.

There are 26 lots within the Big Sky Village (BSV) Subdivision (Fig. 4 pg. 10). Of the 26 lots within the subdivision, 13 have water-wells and there are three adjacent lots that also have wells. Table 5 summarizes information about the subdivision and includes the parcels that border the subdivision.

Table 5. Big Sky Village Subdivision and surrounding parcels, ownership and details.

Lot	Owner	Well Number	Permitted diversion, ac-ft/yr
A	Thomas & Jeannie Izzo	N/A	0
B	Roty LLC	N/A	0
C	Quantum Lizard LLC	LRG 13218	1
D	Brent & Melissa Ziarnick	N/A	0
E	Ronald Williams	LRG 13413	3
F	Alice Bryan	N/A	0
G	Julie Munoz	LRG 07768	3
H	James Smith	LRG 14839	1
I	Tressa Woolf	N/A	0
J	William & Juanita Crisp	LRG 14829	1
K-1	Bud Walters	N/A	0
K-2	Don Lowe	N/A	0
L	Hazel Johnson	LRG 07763-S	TBD
M	Maxine Thurman	N/A	0
N	Maxine Thurman	LRG 09740	3
O	Neidig & Brouse	N/A	0
P	John & Helen Tate	LRG 14379	3
Q	Freddie Torres	N/A	0
R	Randy Martin	N/A	0
S	Thomas Eungard	LRG 06428	3
T	Bud Walters	N/A	0
U	Evans & Katehakis	LRG 10638	0
V	Evans & Katehakis	LRG 10637	0
W	Tom Wellman	LRG 10640	3
X	Dean Revocable Trust	LRG 13944	3
	Bobby & Daphne Tribble	LRG 09221	3
	Robert Christ	LRG 05854	3
	Cutter Cattle Co.	LRG 04858	15.5
		Total	45.5

As the table shows, there are a number of lots in the subdivision with no well in place. Other lots have wells in place that have not been permitted to divert water because the lots are vacant and the required paperwork must be filed first. Many of these lots are 10 acres in size and have the potential to be divided up further, as Lot K represents. Table 6

compares the total diversions in the BSV Subdivision for present day demand and potential future demand based upon the following assumptions:

1. Only single-household domestic permits of 1 ac-ft/yr are issued for lots with no well or no assigned diversion.
2. The TBD value was assigned 16.5 ac-ft/yr water rights.
3. Lots sizes will remain the same preventing any further subdividing.

Table 6. Current diversions vs. future diversions, Big Sky Village Subdivision and surrounding lots.

Current total permitted diversions, ac-ft/yr	Future total permitted diversions, ac-ft/yr
45.5	76.0

While the information in Table 6 does represent what is, and possibly could be, appropriated in the Cutter area, current use of water is considerably less. As discussed previously, the amount of water used by individuals in such a rural setting is debatable. Estimates average around 100 gpcd. Using the averaged metered use of 210 gpcd from the data related to Mr. Smith (LRG-14839), the following information was calculated.

Current population near the Cutter area around 40 people and the majority of the population resides within the boundaries of the aquifer tapped by LRG-10140. Looking at Figure 4 (pg. 10) shows that the BSV Subdivision makes up the main concentration of the Cutter population with a few other developed parcels within the aquifer boundary. For this reason, a population of 25 people was used to calculate current water use for the community. For estimated future use, it was assumed that all of the lots within the

subdivision had homes with an average of 2.5 residents per home. In addition to these estimates, diversions from the surrounding irrigation wells previously mentioned were added in full. Table 7 summarizes the demand for water based upon these assumptions.

Table 7. Current and future water use for the area of Cutter within the assumed aquifer boundary.

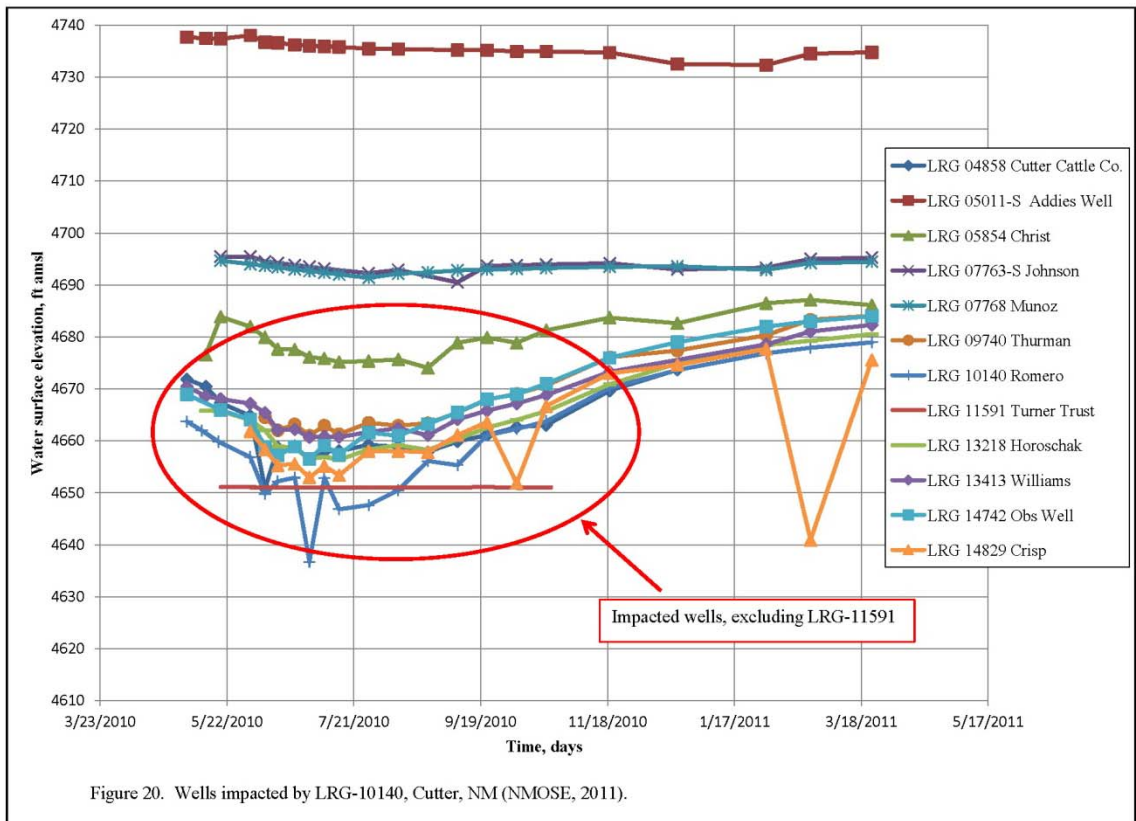
Current water use				Future water use			
Population	Use, gpcd	Total, gal/yr (domestic use)	Total, ac-ft/yr (plus irrigation rights)	Population	Use, gpcd	Total, gal/yr (domestic use)	Total, ac-ft/yr (plus irrigation rights)
25	210	1,916,250	40.9	60	210	4,599,000	49.6

Values are significantly impacted by irrigation rights like those associated with wells LRG-07768, LRG-07763, and LRG-04858. If future irrigation rights are issued within the aquifer boundary they will create greater impacts.

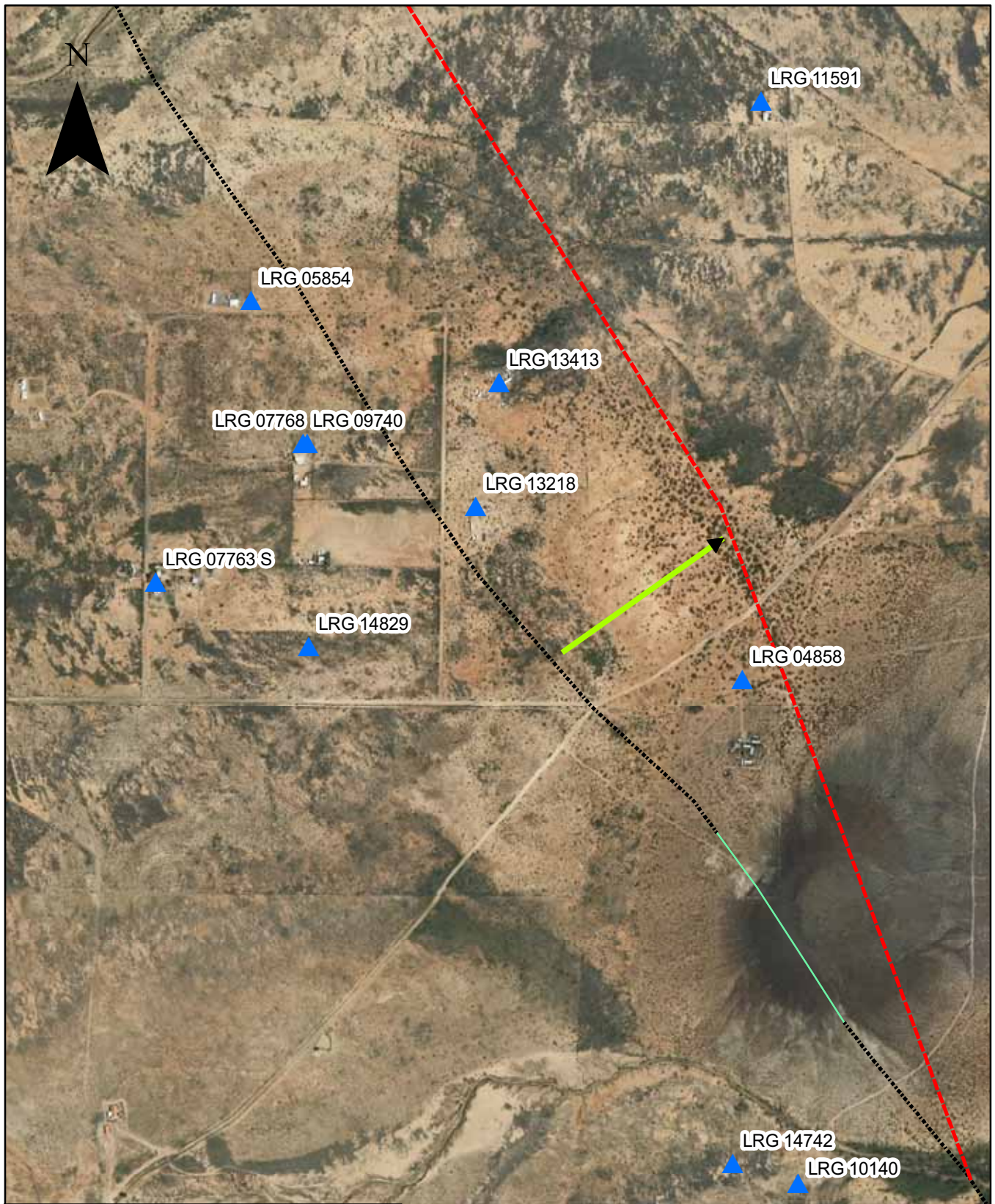
Estimated Aquifer Extent





In order to determine a storage coefficient for the aquifer that lies beneath Cutter, the extent of the aquifer is needed. While the exact area of the aquifer is not known, studying area wells and water level measurements can give an estimate to the extent of this system. The geologic characteristics, discussed earlier in this paper, were also used to set boundaries for the system. What resulted was an estimate for the extent of the aquifer tapped by LRG-10140.

Water levels were the first clue as to what wells in the community are connected to LRG-10140. There are definite impacts on wells LRG-4858, LRG-5854, LRG-9740, LRG-13218, LRG-13413, LRG-14742, and LRG-14829, are observed. Wells showing little to no response to pumping include LRG-5011-S, LRG-7763-S, LRG-7768, and LRG-11591 (Fig. 20, pg. 35). While LRG-7763-S, and LRG-7768, show little impacts from pumping, they are considered to be within the aquifer system. The absence of significant drawdown in these two wells can be attributed to their shallow depths, which are listed in Table 3. LRG-11591 showed no signs of drawdown impacts and is not connected because of the fact that it lies east of the Jornada Draw Fault boundary.



While the exact location of the Jornada Draw Fault is not known, evidence from this study suggests that it lies to the east of LRG-4858 and LRG-13413, and to the west of LRG-11591. Original mapping, from the USGS, has the Jornada Draw Fault running in northwestern direction from the western side of Black Hill. However, wells that responded to pumping suggest the fault may be located farther to the east (Fig. 21, pg.36). The depth of the water table in LRG-11591 coupled with the constant water level help to confirm the fault location (Fig. 20).



Explanation	
	Monitored Wells
	Jornada Draw Fault Adjusted
Jornada Draw Fault	
	Inferred
	Well constrained

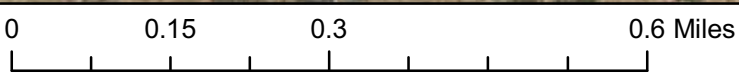
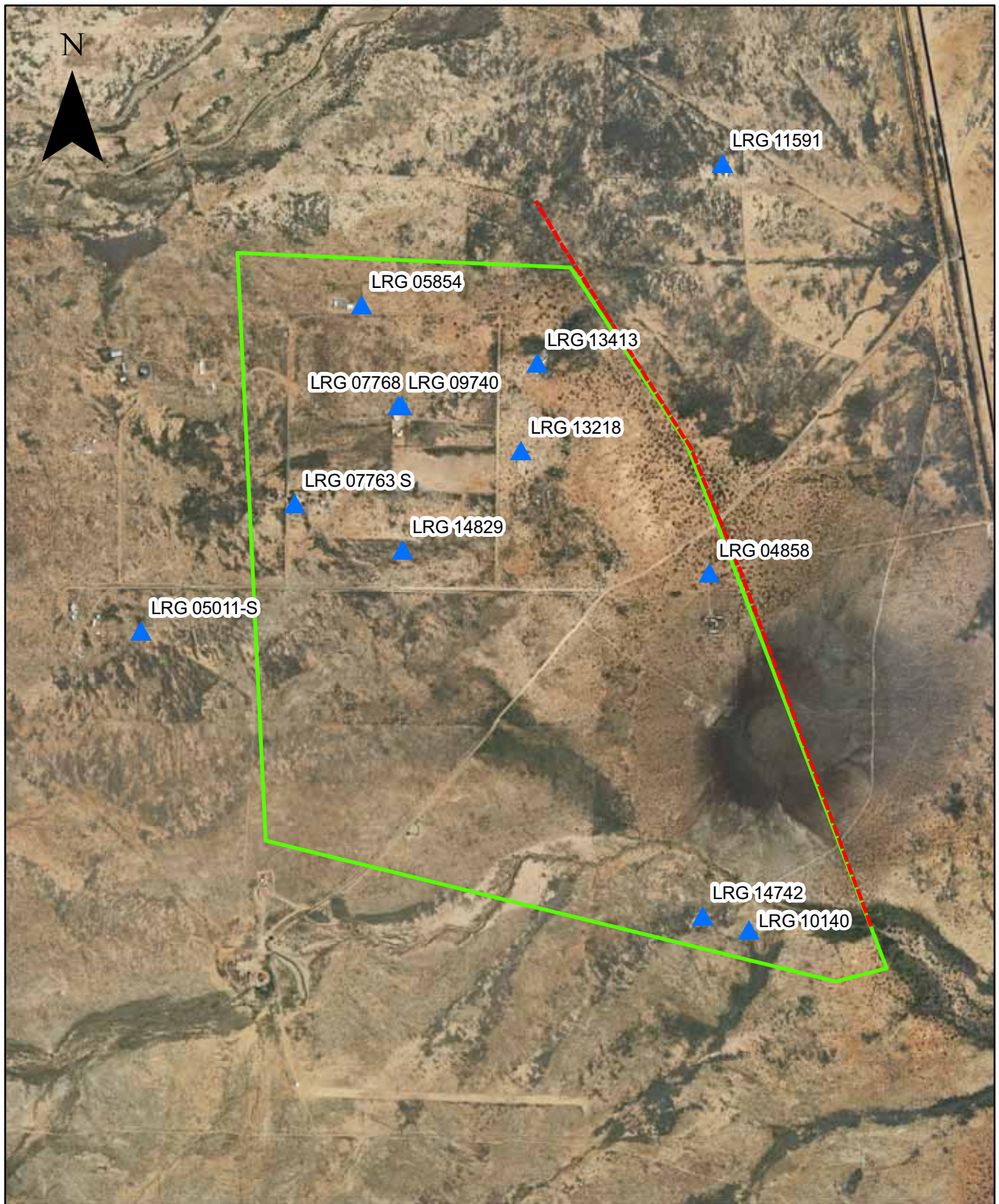





Figure 21. Map showing location of Jornada Draw Fault, Sierra County, New Mexico

The Jornada Draw Fault sets the eastern boundary for this system. The southern border is determined by the Tertiary-age igneous dikes mentioned in the JSAI, 2009B, report (Fig. 2 pg. 7). The western boundary lies east of LRG-5011-S as indicated by the water level data. The northern boundary extends to LRG-5854 due to the fact it is the farthest northern well with available data. Figure 22 (pg. 38) shows a map of the aquifer area based upon the above information. Table 8 summarizes the spatial attributes.

Table 8. Summary of spatial attributes for the aquifer tapped by LRG-10140.

Area of aquifer, m ²	Area of aquifer, ft ²	Area of aquifer, acres
2,706,633	29,133,960	669



Explanation	
	Monitored Wells
	Jornada Draw Fault Adjusted
	Estimated Aquifer Area

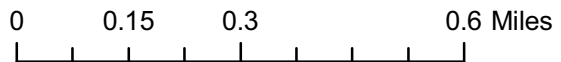


Figure 22. Map showing estimated area of the aquifer system beneath Cutter, NM

Storage Coefficient

In order to determine the effects of population growth on the aquifer beneath Cutter, a storage coefficient was calculated. Since the system is unconfined, Fitts, 2002, approach to specific yield was used to determine this coefficient. The formula is listed below.

Equation 1. Fitts, 2002, Specific yield equation.

$$dV_w = -S_y A dh$$

Where: dV_w = the amount of water removed
 S_y = specific yield
A = area
dh = change in head

Solving for S_y results in the following:

$$S_y = \frac{dV_w}{A dh}$$

Values of water use were selected from the data included in previous sections. To determine a value for drawdown (dh), the hydrograph (Fig.13 pg. 21) for LRG-14742 was analyzed. From the hydrograph, the water level from July 15, 2010, of 82.85 ft bgl was selected. Taking the drawdown over this period, dh = 49.12 ft. The total volume pumped on that date is 161.948 ac-ft (dV_w). Using these values and the combining them

with the estimated aquifer extent, the storage coefficient was calculated. Table 9 shows the result of the calculation.

Table 9. Specific yield value for estimated aquifer area.

Area, acres	dh, ft	dV_w, ac-ft	Sy
669	49.12	161.948	0.00493

The unconfined system beneath Cutter exhibits a storage coefficient value usually seen within confined systems. While the specific yield approach may not be the most accurate way of calculating an exact storage coefficient for this system, it does help to quantify the secondary porosity that exists in the fractures of the sandstone units. The primary porosity of the system is negligible due to its extremely low value.

Recharge

Recharge was estimated at 80 ac-ft/yr (50 gpm), in the JSAI, 2009B report. This number was calculated by JSAI using modeling software and is considered the best estimate for recharge to the system beneath Cutter. Judging from the recovery water level data on the study wells, the recharge is greater than what is currently being pumped (40.9 ac-ft/yr). Once LRG-10140 was shut off, the water levels in all the study wells began to recover. The value for current actual diversions from the aquifer is 25 gpm. The recharge value of 50 gpm matches the recovery trends in the study wells.

Analysis and Results

The main focus for this study was to determine the impact of growth on the aquifer system beneath Cutter. While there is no exact way to determine how many people will eventually move to the area due to the Spaceport, the methodologies described previously do provide accurate background for projections. The following section describes how the methodologies were combined to understand current and future impacts on groundwater resources beneath Cutter.

Estimated Drawdown

Taking the information from Table 7 (pg. 34), a current demand of 40.9 ac-ft/yr and a future demand of 49.6 ac-ft/yr were selected. The recharge value of 80 ac-ft/yr was included. A mass-balance calculation was performed to account for the amount of water (dV_w) coming into ($Q_{recharge}$) storage (S) and the amount of water being removed (Q_{pumped}). The resulting volume of water calculated in the mass-balance equation was then entered into the specific yield equation and solved for dh (change in head). Results formulated using Equation 2 and 3 are shown in Table 10

Equation 2. Mass-balance equation solved for groundwater system.

$$S \frac{dV_w}{dt} = Q_{recharge} - Q_{pumped}$$

Equation 3. Fitts, 2002, Specific yield equation solved for change in head.

$$dh = \frac{dV_w}{-Sy A}$$

Table 10. Drawdown based upon current and future demand for water.

	Demand, ac-ft/yr	dV_w, ac- ft/yr	dh, ft/yr
Current	40.9	+39.1	+11.9
Future	49.6	+30.4	+9.2

Based upon the above calculations, the aquifer beneath Cutter will recover at a rate of 11.9 ft/yr at current demand and 9.2 ft/yr at future demand. What is apparent is the system's susceptibility to larger diversions seen with irrigation rights. Future irrigation diversions will create greater impacts on recovery and drawdown levels.

Hydrogeology

The system beneath Cutter is best described as a fractured tub. Recharge from the northwest flows south to southeast and is captured by the dikes to the south and the Jornada Draw Fault to the east. Water levels in this system will continue to recover until they reach levels above the boundaries. Once the water in the system fills the fractures above the sandstone units, it then leaks out of the system. Otherwise, water levels would continue to rise and Cutter would be a lake.

Given the above information, the aquifer is heavily dependent on recharge. Years of drought like 2011, greatly impact the aquifer and its ability to sustain a community. Prolonged drought will only curb water level recovery and growth for this area.

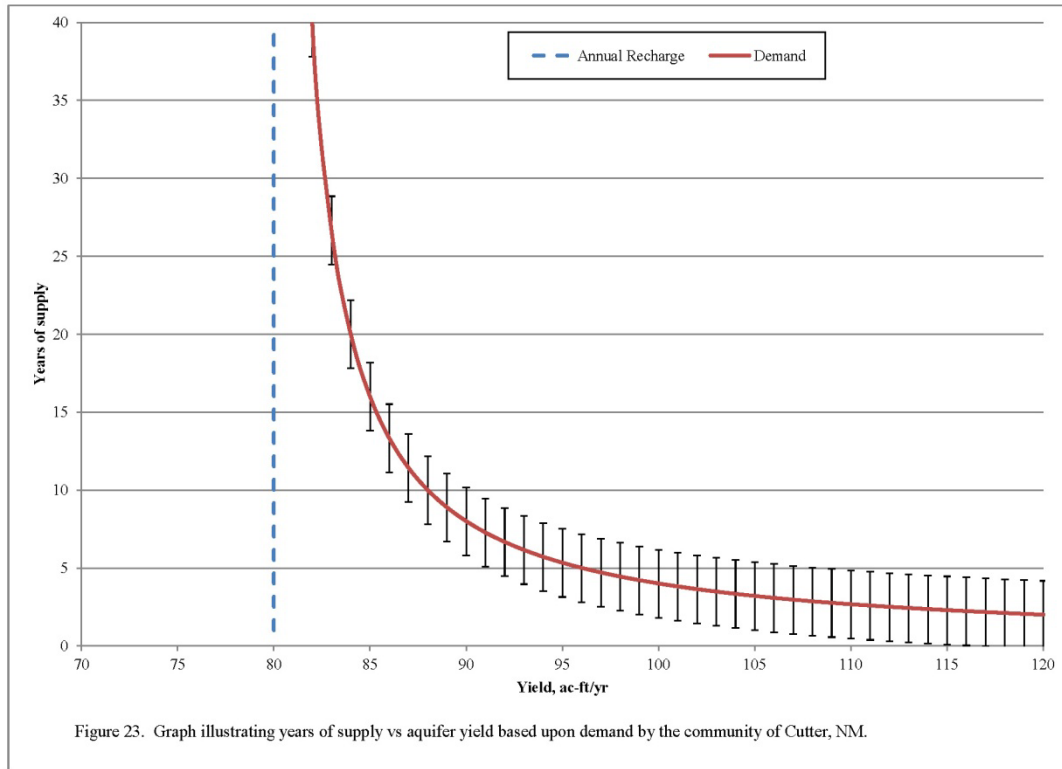
Water Availability

While the BSV subdivision is currently the only subdivision within the aquifer, future development could bring more growth than to just these selected lots. The storage value is incredibly low, meaning that the major factor for water availability is recharge. If demand in the focus area exceeds 80 ac-ft/yr, the aquifer will be mined. Once the aquifer is mined, the sustainability of the system decreases. Table 11, provides calculated drawdown based upon a range of demands.

Table 11. Estimated drawdown based upon a range of demands.

Population	Demand, gpcd	Irrigation rights, ac-ft/yr	Total demand, ac-ft/yr	dh, ft/yr
100	210	35	58.52	6.51
200	210	35	82.05	-0.62
300	210	35	105.57	-7.75

The calculations above indicated that the breaking point for the community population is around 200 people. This is assuming that no further irrigation, industrial, or livestock rights are to be permitted within the area. Figure 23 is a graphical representation of aquifer yield versus years of supply in the aquifer. As the figure illustrates, when the demand approaches the recharge constant of 80 ac-ft/yr, the years of supply increase towards infinity. The opposite is apparent when the demand greatly exceeds recharge. The error bars represent the uncertainty surrounding the analysis.



Uncertainty

The analyses and conclusions drawn from this project do come with some uncertainty. While there was a substantial amount of data provided, assumptions were required to determine the aquifer extent. The northern boundary of the aquifer was assumed to reach only as far as the available water-level data. If the boundary extends farther to the northwest than suggested in this study, the storage coefficient will decrease. Further research into the extent of the system beneath Cutter will only help to better estimate impacts from future development.

Conclusions and Recommendations

Conclusion

It is evident from the data and analyses presented in this project that the aquifer system beneath the community of Cutter is limited. With Spaceport America near completion and further development rumored to be in the works, planners need to take a hard look at the groundwater resources available for this development. The estimates for growth put forth in this project represent the potential of impacts on groundwater resources for this community. Water levels will drop if demand exceeds the recharge.

The data provided from pumping related to construction at the Spaceport gives a strong indication of what the future may hold for this area. The fact that Cutter is one of the only communities within close proximity to the Spaceport indicates a strong potential for growth due to those employed at the facility. While time will only tell the true amount of economic impact the Spaceport will have, the availability of water resources may prove to be the ultimate factor in determining growth for the community of Cutter.

This analysis finds that the aquifer beneath Cutter on the west side of the Jornada Draw Fault could possibly support a community of 200 people. Mining of the aquifer will occur with the demand associated with this population. Drawdown over a 40 year horizon with this demand will lower the water table by 25 feet.

It should be reiterated that this analysis only takes into account the available water resources within the estimated boundary. Areas of Cutter on the east side of the Jornada Draw Fault may provide for further development than what is put forth in this analysis.

Recommendations

The following recommendations are provided to help alleviate problems that may occur from impacts due to drawdown in the aquifer system beneath Cutter.

Solution 1 Deepen domestic wells

Many of the wells in the community of Cutter are shallow. If drawdown impacts force homeowners to replace wells, the new wells should be drilled to depths closer to that of the Romero Well. A negative aspect of this solution is cost. The deeper the well the more it will cost to drill and equip the well. Due to this factor, it may not be a possible solution for some individuals.

Solution 2 Shared domestic wells

Sharing a domestic well between multiple residences can have its advantages. The cost of drilling a larger, deeper well, as mentioned above, is more obtainable because the costs are divided among more individuals. Also, maintenance costs are also divided among more individuals. Downsides to this recommendation are that the shared responsibility is not always as shared as it should be. Many agreements on a shared well can just be verbal in nature and make collecting payment from other parties difficult.

Solution 3 Use the Romero Well to serve as a municipal supply

LRG 10140 has proven itself to be a well capable of supplying water to the community of Cutter. It might be possible to purchase the well and water rights from the current owner and use the well as a municipal water supply. Infrastructure would have to be developed

and the costs for this solution would be large. However, this would help to ensure a reliable water supply and prevent any hardships incurred by individuals on private domestic wells due to drawdown in the aquifer.

Solution 4 Limit development in the area

Limiting permits for agriculture and industrial development within the study will help to offset impacts from drawdown. These areas of economic development will increase demand for water more greatly than growth associated with residential development. Permits for this type of development would be best appropriated outside of the estimated system.

Solution 5 Require conservation

If the community of Cutter wants to plan for the future, smart water conservation practices should be a prominent part of the planning. The aquifer will support more development if household use is reduced. Low flow toilets, low water use landscaping, and rain water harvesting, are just a few examples of ways to reduce water use.

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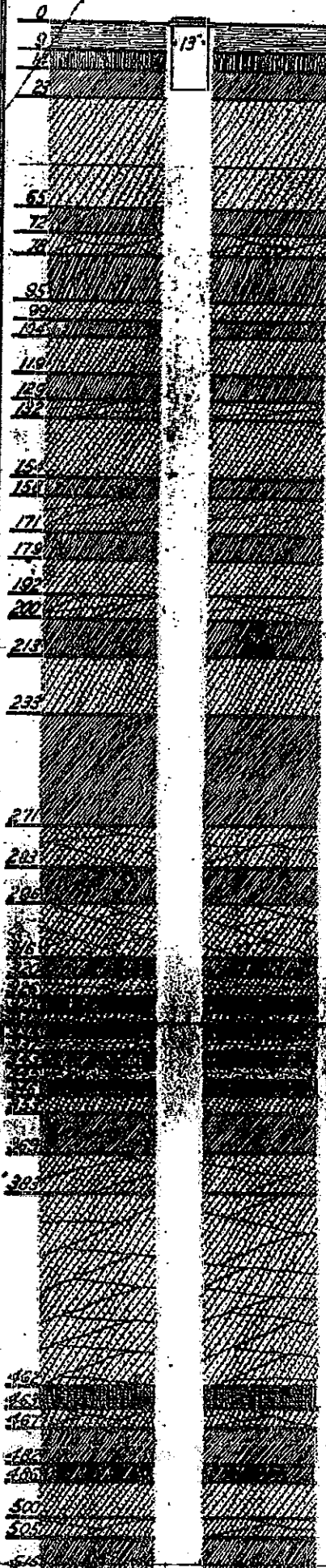
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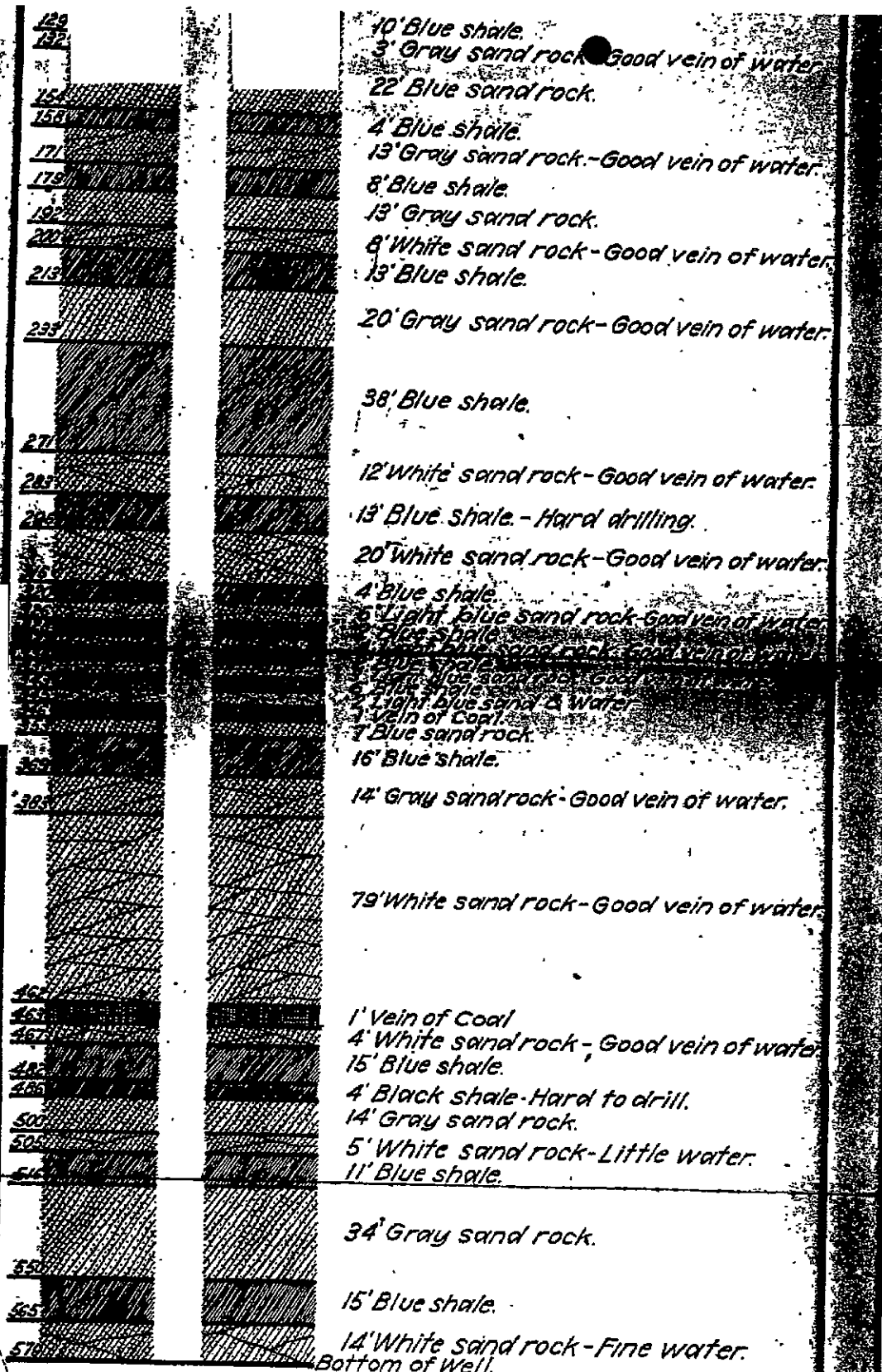
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Appendices



9' Brown hard clay
 5' Diorite.
 9' Blue shale. - Good drilling.
 42' Gray sand rock. - Hard drilling.
 48' Normal water level.
 7' Blue shale
 6' Gray sand rock - Struck first water
 17' Blue shale.
 4' Gray sand rock.
 5' Blue shale.
 15' Blue sand rock.
 10' Blue shale.
 3' Gray sand rock Good vein of water
 22' Blue sand rock.
 4' Blue shale.
 13' Gray sand rock - Good vein of water.
 8' Blue shale.
 13' Gray sand rock.
 8' White sand rock - Good vein of water.
 13' Blue shale.
 20' Gray sand rock - Good vein of water.
 38' Blue shale.
 12' White sand rock - Good vein of water.
 13' Blue shale. - Hard drilling.
 20' White sand rock - Good vein of water.
 4' Blue shale.
 6' Light blue sand rock - Good vein of water.
 2' Blue shale.
 4' Light blue sand rock - Good vein of water.
 2' Blue shale.
 2' Light blue sand rock - Good vein of water.
 6' Blue shale.
 2' Light blue sand & water.
 3' Vein of coal.
 7' Blue sand rock.
 16' Blue shale.
 14' Gray sand rock - Good vein of water.
 79' White sand rock - Good vein of water.
 1' Vein of Coal
 4' White sand rock - Good vein of water.
 15' Blue shale.
 4' Black shale - Hard to drill.
 14' Gray sand rock.
 5' White sand rock - Little water.
 11' Blue shale.



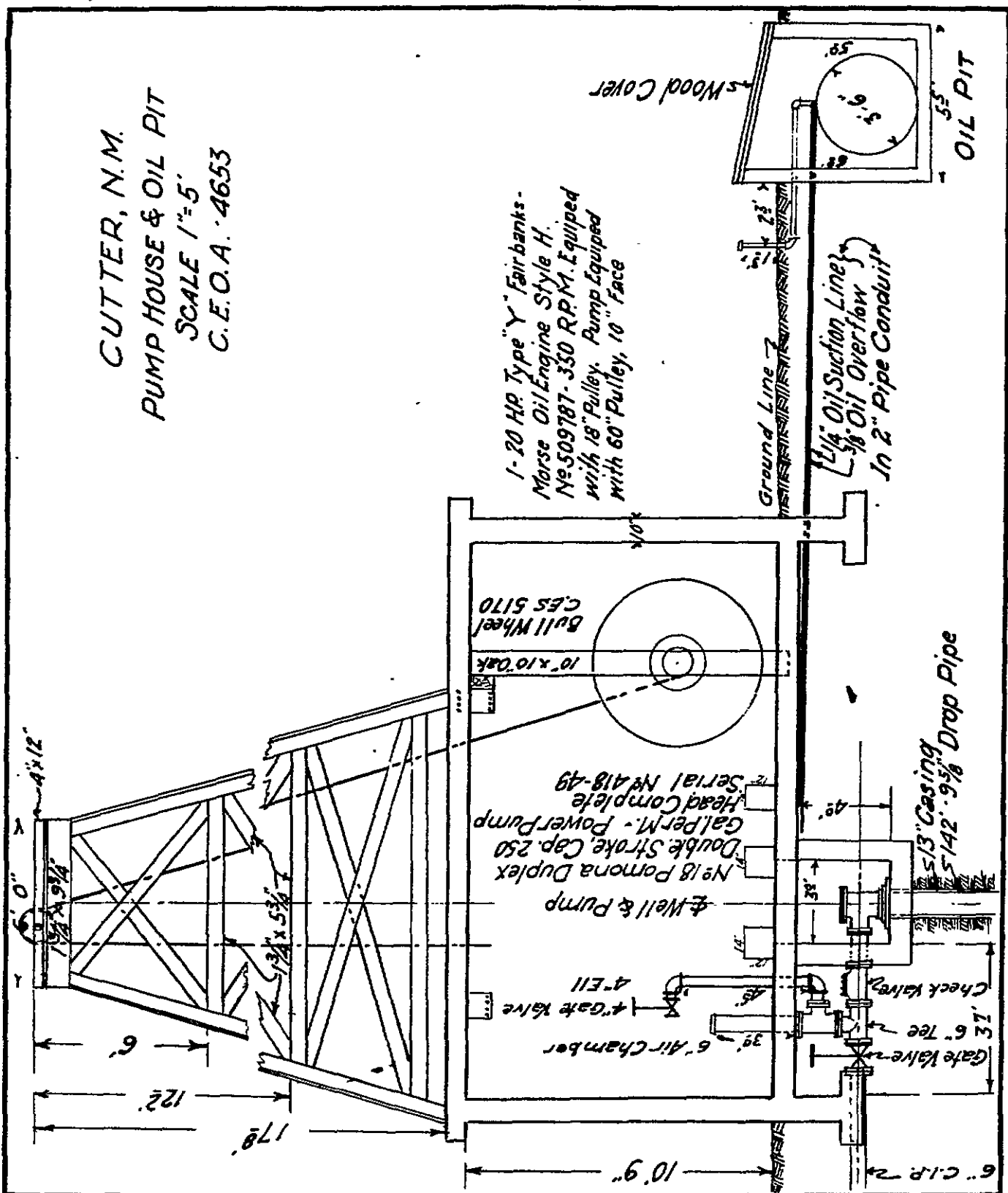
Note:-

1-20' Joint of 13" pipe, top of pipe 12' above level of ground; run in 3" of cement around 13" pipe. Top of cement 4' 06" below top of ground. 13" pipe plugged.

Tested well with a 4 1/2" x 60" single acting working barrel at a depth of 283'. Pumped clear all the time no sand. After 28 hrs. of straight pumping there was 18" of fine sand and drill cuttings settled in bottom of well. Well pumped 55 gallons in 1 minute 8 seconds. Draw down 2'-10".

All measurements taken from Derrick floor, which is 18' above ground.

CUTTER, N.M.
 PUMP HOUSE & OIL PIT
 SCALE 1"=5'
 C.E.O.A. 4653



THE A. T. & S. F. RY SYSTEM
 MAIN LINE TRAFFIC DENSITY IN MILLIONS OF GROSS TONS
 EXCLUSIVE OF LOCOMOTIVE

WESTERN LINES

DIVISION	FACILITY	1935		1936		1937		1938		1939		1940		1941		1942		1943		1944		
		Freight	Pass.	Freight	Pass.	Freight	Pass.	Freight	Pass.	Freight	Pass.	Freight	Pass.	Freight	Pass.	Freight	Pass.	Freight	Pass.	Freight	Pass.	
WESTERN	Newman	2,164	1,726	3,690	2,502	2,532	1,620	4,152	2,200	1,484	3,136	1,497	2,237	2,237	1,497	2,237	1,497	2,237	1,497	2,237	1,497	2,237
	Hutchinson	1,861	1,725	3,556	2,050	2,026	1,700	3,706	1,671	1,536	3,153	1,802	1,802	1,802	1,802	1,802	1,802	1,802	1,802	1,802	1,802	1,802
	Hutchinson	1,723	1,365	3,008	1,940	2,009	1,283	3,240	1,690	1,019	2,883	1,934	1,934	1,934	1,934	1,934	1,934	1,934	1,934	1,934	1,934	1,934
COLORADO	La Junta	1,429	1,314	2,743	1,319	1,498	1,317	2,815	1,601	1,053	1,654	1,307	1,250	1,250	1,250	1,250	1,250	1,250	1,250	1,250	1,250	1,250
	Doña Ana	1,641	1,726	3,690	2,502	2,532	1,620	4,152	2,200	1,484	3,136	1,497	2,237	2,237	1,497	2,237	1,497	2,237	1,497	2,237	1,497	2,237
	Hutchinson	1,861	1,725	3,556	2,050	2,026	1,700	3,706	1,671	1,536	3,153	1,802	1,802	1,802	1,802	1,802	1,802	1,802	1,802	1,802	1,802	1,802
NEW MEXICO	Albuquerque	1,061	1,459	2,502	1,319	1,392	1,479	2,871	1,057	1,313	2,370	1,300	1,250	1,250	1,250	1,250	1,250	1,250	1,250	1,250	1,250	1,250
	Las Alamos	1,040	1,552	2,592	1,158	1,170	1,502	2,922	1,191	1,421	2,613	1,371	1,384	1,755	1,485	1,358	2,843	1,918	1,307	1,356	1,952	1,434
	Las Alamos	1,040	1,552	2,592	1,158	1,170	1,502	2,922	1,191	1,421	2,613	1,371	1,384	1,755	1,485	1,358	2,843	1,918	1,307	1,356	1,952	1,434
PENNSYLVANIA	Washington	1,308	1,429	2,743	1,319	1,392	1,479	2,871	1,057	1,313	2,370	1,300	1,250	1,250	1,250	1,250	1,250	1,250	1,250	1,250	1,250	1,250
	Washington	1,308	1,429	2,743	1,319	1,392	1,479	2,871	1,057	1,313	2,370	1,300	1,250	1,250	1,250	1,250	1,250	1,250	1,250	1,250	1,250	1,250
	Washington	1,308	1,429	2,743	1,319	1,392	1,479	2,871	1,057	1,313	2,370	1,300	1,250	1,250	1,250	1,250	1,250	1,250	1,250	1,250	1,250	1,250

Note: Sequence of towns in each case in second column is from east towards west (west bound, time table direction).
 * W.B. equals S.B. of Employees' time table.
 (Note: No tonnage figures available for D. & R. W. between Denver and Brighton)
 2. Exclusive of C. & S. trackage between Pueblo and Denver (117 miles), which tonnage (in millions of gross tons) is as follows:
 Freight, Passenger Total
 1935: 1,477 1,837
 1936: 1,838 2,186
 1937: 2,177 2,552
 1938: 1,393 1,751
 1939: 1,856 2,181
 (Note: No tonnage figures available for D. & R. W. between Denver and Brighton)
 3. Exclusive of C. & S. trackage between Pueblo and Denver (117 miles), which tonnage (in millions of gross tons) is as follows:
 Freight, Passenger Total
 1935: 1,247 1,604
 1936: 1,487 1,835
 1937: 1,673 2,139
 1938: 1,118 1,474
 1939: 1,318 1,661

THE A. T. & S. F. RY. SYSTEM
MAIN LINE TRAFFIC DENSITY IN MILLIONS OF GROSS TONS
EXCLUSIVE OF LOGGERS

WESTERN LINES

DIVISION	FROM: TO	DI-RECTION	1945		1946		1947		1948		1949		1950		1951		1952		1953		1954		
			FRIGHT	PASS	TOTAL	FRIGHT	PASS	TOTAL	FRIGHT	PASS	TOTAL	FRIGHT	PASS	TOTAL	FRIGHT	PASS	TOTAL	FRIGHT	PASS	TOTAL	FRIGHT	PASS	TOTAL
WESTERN	NEWTON	E.B.	5,501	3,103	8,604	4,542	2,476	7,018	5,988	2,280	8,108	5,259	2,559	7,818	4,551	2,428	6,979	3,247	2,335	5,584	2,972	2,370	5,342
	RITCHIE	W.B.	3,327	3,057	6,384	2,963	2,437	5,400	3,579	5,817	3,428	3,453	2,973	6,026	3,122	2,475	5,597	3,425	2,430	5,955	2,972	2,370	5,342
	HITCHINSON	E.B.	3,565	2,866	6,431	3,125	1,913	5,038	4,656	1,693	6,349	4,063	2,014	3,379	3,140	1,878	5,018	3,468	1,812	5,280	3,285	1,880	5,165
	HITCHINSON	W.B.	1,194	2,818	4,012	1,363	1,818	3,181	1,519	1,695	3,214	1,437	2,028	3,264	1,317	1,935	3,252	1,542	1,894	3,436	1,745	1,856	3,605
	(W.B. JOHN)	E.B.	5,501	3,103	8,604	4,542	2,476	7,018	5,988	2,280	8,108	5,259	2,559	7,818	4,551	2,428	6,979	3,247	2,335	5,584	2,972	2,370	5,342
COLORADO	DODGE CITY	E.B.	2,421	2,896	5,317	1,877	2,254	4,131	1,966	2,180	4,176	2,407	4,318	2,076	2,334	4,410	2,313	2,323	4,636	2,262	4,445	1,890	4,152
	LA JUNTA	E.B.	3,974	3,802	7,776	3,502	2,643	6,104	3,909	3,929	4,438	3,960	4,484	4,699	4,215	5,226	4,741	4,800	5,384	4,120	4,671	3,725	5,411
	PUEBLO	E.B.	4,059	3,721	7,780	3,353	3,597	6,950	4,019	4,547	4,566	3,768	3,301	3,891	3,628	4,529	4,157	4,269	4,584	3,524	4,250	3,597	5,225
	PUEBLO *	E.B.	1,709	2,895	4,604	1,613	2,616	4,229	1,658	2,528	4,186	1,507	2,500	4,007	1,719	2,511	4,230	1,873	2,500	4,372	1,873	2,511	4,230
	PUEBLO	W.B.	1,642	2,590	4,232	1,395	2,598	3,993	1,495	2,521	4,096	1,208	2,496	3,704	1,495	2,511	4,007	1,873	2,500	4,372	1,873	2,511	4,230
NEW MEXICO	LA JUNTA	E.B.	2,063	2,586	4,649	1,825	2,088	3,913	1,914	3,344	3,648	1,844	2,105	3,949	1,772	1,495	3,267	1,949	2,537	4,066	1,467	1,934	3,401
	ALBUQUERQUE	E.B.	3,437	2,728	6,165	2,074	1,445	3,519	2,190	2,076	4,265	2,231	4,455	2,303	2,103	4,406	2,257	2,255	4,513	2,275	4,290	1,802	4,093
	ALBUQUERQUE	W.B.	2,614	2,558	5,172	1,653	2,119	3,772	1,777	2,103	3,880	1,878	2,178	3,056	1,771	2,335	4,102	2,354	2,354	4,457	1,853	2,236	4,089
	ISLETA	E.B.	2,681	2,549	5,230	1,890	1,182	3,072	2,069	1,886	2,082	1,716	2,258	1,747	2,006	3,752	2,089	1,916	2,285	2,007	1,833	2,190	1,938
	RINGON	E.B.	2,308	2,285	4,593	1,298	1,155	2,453	1,463	1,411	2,866	1,438	1,447	2,881	1,312	1,174	1,486	1,287	1,160	1,864	1,173	1,148	1,321
PANHANDLE	WELLINGTON	E.B.	1,333	2,220	3,553	972	1,171	2,143	1,080	1,157	2,337	1,041	1,440	1,811	1,041	1,355	1,776	1,141	1,160	1,301	1,118	1,130	1,248
	WELLINGTON	W.B.	1,333	2,220	3,553	972	1,171	2,143	1,080	1,157	2,337	1,041	1,440	1,811	1,041	1,355	1,776	1,141	1,160	1,301	1,118	1,130	1,248
	WELLINGTON	E.B.	1,333	2,220	3,553	972	1,171	2,143	1,080	1,157	2,337	1,041	1,440	1,811	1,041	1,355	1,776	1,141	1,160	1,301	1,118	1,130	1,248
	WELLINGTON	W.B.	1,333	2,220	3,553	972	1,171	2,143	1,080	1,157	2,337	1,041	1,440	1,811	1,041	1,355	1,776	1,141	1,160	1,301	1,118	1,130	1,248
	WELLINGTON	E.B.	1,333	2,220	3,553	972	1,171	2,143	1,080	1,157	2,337	1,041	1,440	1,811	1,041	1,355	1,776	1,141	1,160	1,301	1,118	1,130	1,248

1. Exclusive of C. & S. trackage between Denver and Pueblo (117 miles), which average (in millions of gross tons) is as follows:

2,639	1,608	3,247	2,307	1,513	2,820	2,568	3,014	2,571	3,392	2,963	2,462	3,367	2,729	2,332	2,705	2,861	4,414	3,275	2,920	4,414	2,934	2,714	4,420	3,134	2,470	4,423	2,893
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(NOTE: No tonnage figures available for D. & R. G. W. between Denver and Bragdon).

2. Exclusive of C. & S. trackage between Pueblo and Denver (117 miles), which tonnage (in millions of gross tons) is as follows:

2,576	1,582	3,158	1,976	1,490	2,466	2,326	4,436	2,762	2,200	3,324	2,327	2,025	3,374	2,399	2,535	3,396	2,731	2,151	3,394	2,151	3,394	2,032	3,395	2,447	2,736	4,411	2,147
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(NOTE: No tonnage figures available for D. & R. G. W. between Bragdon and Denver).

3. Exclusive of D. & R. G. W. and C. & W. trackage between Trinidad and Jensen (1.8 miles)

NOTE: Sequence of towns in each case in second column is from east towards west (west bound, time table direction).

* W.B. equals S.B. of Employees' time table.